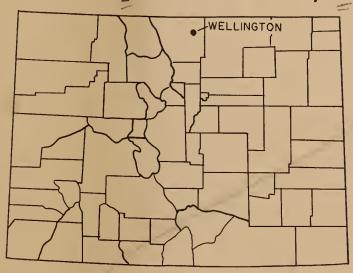
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# FLOOD PLAIN MANAGEMENT STUDY

BOXELDER CREEK IN THE VICINITY OF TOWN OF WELLINGTON, CO



Prepared by the

U.S. Department of Agriculture
Soil Conservation Service;
Denver, Colorado

in cooperation with the Colorado Water Conservation Board,
Town of Wellington,
Larimer County, Colorado. --

September 1983

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This report includes information on the flood hazard areas along Boxelder Creek in the vicinity of Wellington, Colorado.

Because of potential flood damages, detailed flood hazard studies have been recognized as an essential item in guiding the use of flood plains.

The purpose of this report is to provide adequate mapping and data for implementing flood plain management programs.

Included in the report are information on past floods, flood potential, maps, profiles, cross sections, discharge data, and recommendations for reducing potential flood damages in the Town of Wellington.

The Soil Conservation Service conducted the technical studies and prepared the report. These services were carried out in accordance with the Plan of Work of March 1982.

The assistance and cooperation provided by the Colorado Water Conservation Board, Town of Wellington and Larimer County are appreciated and gratefully acknowledged. Financial assistance provided by the Board, the Town and County included funds for photogrammetric maps, and cross section data.

The survey, hydrologic, hydraulic, and other pertinent data and computations are on file with the U.S. Department of Agriculture, Soil Conservation Service, 2490 West 26th Avenue, Denver, Colorado 80217, telephone (303) 837-5653. Additional copies of this report may be obtained from the Colorado Water Conservation Board, the Town of Wellington, Larimer County, or the Soil Conservation Service.

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# FLOOD PLAIN MANAGEMENT STUDY BOXELDER CREEK COLORADO

#### INTRODUCTION

This flood plain management report was prepared by the U.S. Department of Agriculture, Soil Conservation Service, in cooperation with the Colorado Water Conservation Board, Town of Wellington and Larimer County. Interpretations of the flood plain management study and recommendations to reduce damages are included; however, it is beyond the scope of this report to provide specific proposals or plans to rectify the flooding problems.

#### Objectives

The objective of this study is to provide flood plain management information and mapping to the Town of Wellington and Larimer County for use in implementing flood plain management programs which will minimize potential flood losses. Included in the report are engineering and hydrologic data which will facilitate the development of a flood plain management plan, road and bridge plans and design, and non-structural and/or structural flood control measures (if needed).

#### Authority

This study was requested by the Town of Wellington and Larimer County through the Colorado Water Conservation Board (CWCB). The CWCB is the state coordinator for all flood plain information studies and is responsible for setting priorities and scheduling these studies. The CWCB and the Soil Conservation Service entered into a Joint Coordination Agreement for flood hazard analyses in January 1972 (revised November 1978). The Plan of Work for the Study was prepared in March, 1982.

Section 37-60-106(1)(c), Colorado Revised Statutes 1973, authorizes the Colorado Water Conservation Board "to designate and approve storm or flood-water runoff channels or basins, and to make such designations available to legislative bodies of cities and incorporated towns, to county planning commissions, and to boards of adjustment of cities, incorporated towns, and counties of this state." The Board provides assistance to local governments in development and adoption of effective floodplain ordinances. In addition, the Board will provide technical assistance to local entities during the performance of floodplain information studies within Colorado. Presently, financial assistance for the performance of floodplain studies is no longer available from the board.

Section 30-28-111 for county governments and Section 31-23-201 for municipal governments of the Colorado Revised Statutes 1973, states: The cities, incorporated towns, and counties within the study area may provide zoning regulations: "...to establish, regulate, restrict, and limit such uses on or along any storm or floodwater runoff channel or basin that has been designated and approved by the Colorado Water Conservation Board, in order to lessen or avoid the hazards to persons and damage to property resulting from the accumulation of storm or floodwaters..."

Therefore, upon official approval of this report by the Colorado Water Conservation Board, the areas described as being inundated by the 100-year flood (Intermediate Regional Flood)<sup>1</sup> can be designated as flood hazard areas and their use regulated accordingly by the local agencies.

 $<sup>^{</sup>m l}$  The terms "Intermediate Regional Flood," "100-year flood," and "one percent flood" can be used interchangeably as they are all defined by the same type of flood event (see Glossary).

Flood plain management studies are carried out by the Soil Conservation Service as an outgrowth of the recommendations in <u>A Report by the Task Force on Federal Flood Control Policy</u>, House Document No. 465 (89th Congress, August 10, 1966), especially Recommendation 9(c), <u>Regulation of Land Use</u>, which recommended the preparation of preliminary reports for guidance in those areas where assistance is needed before a full flood plain information report can be prepared or where a full report is not scheduled.

Authority for funding flood plain management studies is provided by

Section 6 of Public Law 83-566, which authorizes the U.S. Department of

Agriculture to cooperate with other federal, state and local agencies to

make investigations and surveys of the watersheds and rivers and other

waterways as a basis for the development of coordinated programs.

In carrying out flood plain management studies, the Soil Conservation

Service is being responsive to Executive Order 11988, entitled "Flood Plain

Management", and Executive Order 11990, entitled "Protection of Wetlands"

(both effective May 24, 1977).

#### DESCRIPTION OF THE STUDY AREA

#### Drainage Basin

The Boxelder Creek Watershed covers an area of 251 square miles in parts of Larimer and Weld Counties, Colorado and Albany and Larimer Counties, Wyoming. The watershed is about 32 miles in length and averages about 8 miles in width. Boxelder Creek is joined by Sand Creek, Rawhide Creek, Coal Creek, and Indian Creek before it flows into the Cache la Poudre River about 3 miles southeast of Fort Collins.

The study area lies within the Colorado piedmont section of the Great Plains physiographic province. Elevations range from 7,720 feet at the upper end of the basin to 5,000 feet at the lower study limit. The basin physiography consists of broadly rolling plateaus with hogback ridges and narrow mesas interspersed with narrow valleys and canyons.

#### Climate

The climate is considered semi-arid with normal annual precipitation totaling about 14 inches. About 9 inches of the normal annual precipitation occurs during May through September.

Precipitation is usually generated from cold fronts from the Northwest associated with moisture pushed into the area from the Gulf of Mexico by low pressure systems. Local thunder storms produce some additional summer precipitation.

Temperature data from Fort Collins shows a mean annual temperature of 48 degrees. The average frost-free season is 144 days from May 8 to September 29.

#### Land Use and Soils

Soils in the basin are variable. The northwest portion of the basin includes shallow to moderately deep soils developed on granites. The Western area includes soils associated with sandstone, shale, and limestone bedrock. Moderately deep gravelly soils on small plateau or mesa areas make up most of the northeast portion of the basin. Soils of the irrigated land are predominately deep or moderately deep loams over shales, sandstone or gravels. The larger drainages are predominately alluvial soils of loam to clay loam in texture.

About 67 percent of the basin is forest and range land. The natural cover on the rangelands consist of mixed grassland and shrub plant communities. Ponderosa pine and Juniper trees make up the forested areas. Cropland and hayland make up 27 percent of the basin and the remaining 6 percent is dedicated wildlife and miscellaneous areas.

The Town of Wellington and the communities of Buckeye and Waverly are within the basin.

#### Study Limits

The lower study limit is about 4.5 miles above the confluence of Boxelder Creek with the Cache la Poudre River and extends upstream along Boxelder Creek to a point 1/2 miles north of Wellington, Colorado. The Coal Creek flood plain, from its confluence with Boxelder Creek (1.0 mile south of Wellington) to a point 1/2 miles north of Wellington, is also included in this study. Both Boxelder Creek and Coal Creek flow through the Town of Wellington. These are the only stream reaches within the study area that involve an urban community.

The total length of flood plain included in this study is 9.43 miles along Boxelder Creek and 2.50 miles along Coal Creek.

#### Study Reaches

The study area map was divided into stream reaches because of significant differences in flood plain topography, hydraulic characteristics, and magnitude of discharges.

The enclosed study area map (index map) shows the location of each reach. Reach distances are as follows:

Reach Number	<u>Length-Miles</u>
<ul> <li>1 - Boxelder Creek</li> <li>2 - Boxelder Creek</li> <li>3 - Boxelder Creek</li> <li>4 - Boxelder Creek</li> <li>5 - Boxelder Creek</li> <li>6 - Boxelder Creek</li> <li>7 - Coal Creek</li> <li>8 - Coal Creek</li> <li>9 - Coal Creek</li> </ul>	4.89 1.67 1.03 0.56 0.67 0.61 0.85 1.13 0.52
	TOTAL 11.93 miles

The various exhibits included in this report are related to these reach designations.

#### Natural and Beneficial Flood Plain Values

The flood plains along Boxelder Creek are intensly cropped. Only the main channel has been left in a natural state. This narrow channel is vegetated with a variety of forbs, grasses, sedges and rushes, interspersed with cottonwoods, willows and siberian elm. The meandering channel, passing through intensely cropped farmland, provides a diversity in landscape. This diversity enhances the visual aesthetics and wildlife habitat values in the area.

Although narrow, the channel corridor provides a travelway and cover for wildlife. It is used primarily by mule deer, ringnecked pheasants, mourning doves, cottontails and some migratory waterfowl. Cropland in the flood plain supply a food source for wildlife in the area.

A small irrigation regulating reservoir is located next to Boxelder Creek approximately one-half mile below Wellington. Other small areas of open water occur in the Boxelder Creek channel. While not within the study area, there are numerous irrigation supply reservoirs throughout the general area. These reservoirs are used extensively by migratory waterfowl and resident Canada geese.

of particular interest to wildlife and sportsmen groups is the 1,242 acre Wellington State Wildlife Area located along the Indian Creek flood plain southeast of Wellington, Colorado. This is a predominantly wet area where ground water is forced to the surface by tighter soils and shallow depths to shale. Cover and food plots have been put in for waterfowl and game birds. The Wellington Wildlife Area provides good pheasant, duck and rabbit hunting during those seasons. Sportsmen and others may be use the area at any time for hiking, hunting, bird watching or other diversions. No parking or camping is permitted within the area.

#### RELATED FLOOD STUDIES

Five floodwater retarding reservoirs were constructed during the period 1971 through 1981. Four are located in the basin upstream from the upper study limit, and the fifth is on Indian Creek which joins Boxelder Creek near the middle of the study area, see figure 1. The structures were built under the Watershed Protection and Flood Prevention Act (Public Law 566, 83rd Congress). A planning document "Watershed Work Plan, Boxelder Creek Watershed" is available at the Soil Conservation Service Office in Denver. The planning for this project considered flood damages for before and after project conditions. The planning detail was not considered adequate, however, for developing zoning ordinances and other regulation measures needed for managing the flood plain.

A dam breach study was made by the Soil Conservation Service in 1980. The purpose of the study was to define the flood zone of a possible failure of floodwater retarding dam B-2 (see figure 1), which is a part of the Watershed project previously discussed. The report shows the potential dam breach flood boundary from structure B-2 (about 8 miles upstream of Wellington) to the southern boundary of the Town of Wellington.

A master drainageway planning study was made by Simons, Li & Associates, Inc. in 1981. The study encompassed cooper slough and the reach of Boxelder Creek downstream from the lower study limit of this flood plain management study. Hydrologic information from the Simons & Li Report was used in this study.

#### FLOOD HISTORY

Flooding on Great Plains streams in Colorado is caused by summer storms. Colorado Front Range streams flood from summer rains as well as from snowmelt and occasionally from a combination of the two. The Boxelder Creek Basin is more characteristic of the Great Plains streams because of its orientation and elevation. The relatively recent "Big Thompson River Flood of 1976" is an indication of the magnitude of rainfall (12 in.) that can occur in this part of Colorado. This was a July 31-August 1 storm.

According to the "Watershed Work Plan, Boxelder Creek Waterhshed", damaging floods occurred on Boxelder Creek on an average of once every year. Because of the recentness of reservoir construction (project completed 1981), there is no flood history of the study area for post project conditions. The reservoirs retain runoff from a combined area of 175.5 square miles or 70 percent of the drainage basin. The level of flood protection decreases, however, as the distance below the structures increases. The project work plan estimated that at Highway 14, east of Fort Collins, the pre-project 100 year flood would be reduced by the project to a 4 year frequency.

The above information however is based on uniform rainfall over the entire basin. This is generally not the way storms occur historically. Flooding from moving storm cells can occur along any portion of the basin, including the area uncontrolled by reservoirs, therefore, records of past flood events in the basin are still useful information. A discussion of recorded flooding follows.

Boxelder Creek has a long history of flooding. Floods of record occurred in 1969, 1967, 1965, 1961, 1947, 1933, 1930, 1922, 1909, and 1904. There is no official gauging station in the watershed and no known flood flows have been measured. In general, information on past floods is based on newspaper accounts and interviews with residents of the area.

The earlist known flood was in 1904. The flood of May 20 and 21, 1904, resulted from rains of "cloudburst intensity" in the foothills at about 7,000 feet elevation on the headwaters of the North Fork Cache la Poudre River and Boxelder Creek. Damage was heavy at Fort Collins and Boxelder Creek, downstream of Fort Collins, Colorado, contributed high flows. The Greeley Tribune stated:

"The Boxelder, a small stream ordinarily only a few feet wide, was tearing down through a fertile valley filled from bluff to bluff with a sheet of water a mile wide, carrying buildings and bridges away..."

Information obtained in August 1969 from landowners in the flood plain for the years 1959 through 1969 indicate that flood damage occurs somewhere in the watershed each year. Estimated damage from these storms follows:

August 1, 1961; one 50- to 100-year frequency storm in the vicinity of Wellington, Colorado. Forty basements were flooded in addition to agricultural and non-agricultural crops and property, with damages at \$76,150.

June 1963; one 25- to 50-year frequency storm over a very small area. Estimated damages, \$7,200.

June 14 through 17, 1965, one 2-year, two 1-year, and one 25-year frequency storms with some overlapping of areas. Estimated damages, \$96,000.

May 30 and June 4, 1967; two 25-year frequency storms overlapping over a relatively small area in the vicinity of Wellington, Colorado. Estimated damages of \$46,100 with four lives lost at a county road bridge washout.

August 4, 1969; a 1-year frequency storm occurring over a small area.

Damage estimated at \$4,000.

#### INVESTIGATIONS AND ANALYSIS

#### Interpretation and Use of Report

#### A. Frequency and Discharge

The 10-, 50-, 100-, and 500-year flood events are used as the flood frequencies for this flood plain analysis. Thus the data developed in this report will be compatible not only for regulation purposes, and H.B. 1041 designation but also for Federal Insurance Administration flood insurance rate studies.

These various flood events have an average occurrence of once in the number of years as indicated. For example, the 100-year flood occurs, on the average, once in a 100 year period, and has a one percent chance of being equaled or exceeded in any given year.

The particular uses for the various flood events in addition to those stated above are as follows:

#### 10-Year and 50-Year Flood Events

Information regarding these lower frequency floods is especially useful for future engineering studies and land use planning purposes related to minor road systems, minor channel improvements, the location of parks and recreational facilities, agricultural lands, and appurtenant structures. For structures and uses of this type on the smaller tributaries or in areas where the high risk of structural failure is economically feasible, and the hazard of life and property nonexistent, the use of the lower frequency floods may be considered.

#### 100-Year Flood Event

The 100-Year flood event may also be used for engineering design purposes where a lower risk of failure than the 10- or 50-year flood is desired. However, the most important use of the 100-year flood event lies in flood plain management and land use planning as set forth in the state statutes. The State of Colorado considers the 100-year frequency flood as the flood event to be used in designing and protecting structures and dwellings for human occupation. Therefore, all flood plain regulations are based upon the 100-year flood.

#### 500-Year Flood Event

The 500-year flood event is useful in making the public aware that floods larger than the 100-year flood can and do occur. Just because a person is living above the 100-year flood boundary does not mean that he is completely safe from flooding. The 500-year flood event can also be used for regulating high risk developments within the flood plain such as nuclear power plants, or the storage or manufacture of toxic or explosive materials.

#### B. Flood Elevation

The exhibits and tables display study results by stream reach, as shown on the index map. Flood crest elevations for the 10-, 50-, 100-, and 500-year floods as determined at each cross section may be found in the Flood Frequency-Elevation and Discharge Data table, Table 1. The Cross Section Exhibits, B-1 through B-9, show a graphical representation of the high water elevations at typical valley cross sections throughout the study reach. Water surface elevations at the cross sections were used to prepare the flood profiles, exhibits A-1 through A-9a, which show the streambed elevation in relation to high water elevations for the 10-year 50-year, 100-year, and 500-year frequency floods.

The Flood Profiles may be used in areas where controversy arises over the 100-year flood boundary shown on the Flooded Area plates. Since the Flood Profile plates give the water surface elevation at a specific point on the reference line, the high water elevations can be surveyed on the ground to alleviate any discrepancies on the base map.

#### C. Flooded Areas

The Flooded Area maps, plates 1 through 13, show the boundary of the 100-year and 500-year flood plains. The flood plain boundaries were plotted from the flood profiles by determining the channel stationing of flood contours at the same interval as the topographic maps. Flood contours, shown as wiggly lines, extend perpendicular to the direction of flow and intersect the ground at the edge of the flood plain.

The areas included within the flood line boundaries are about 1,246 acres for the 100-year frequency and 1,444 acres for the 500-year.

Upon official approval of this report by the Colorado Water

Conservation Board, the area outlined by the 100-year flood boundary may be regulated accordingly by the local officials.

#### D. Floodway

Encroachments on flood plains, such as artificial fill, can reduce the areal extent of a flood plain if provisions are made for increased flood heights. As an alternative to the present flooding situation a possible floodway with encroachment dikes was analyzed in this study. This was simply a hydraulic analysis in which the flood plain was theoretically modified to contain flooding within selected encroachment boundaries. The resulting effects on flood elevations are shown in an Appendix separate from this report.

#### Hydrology

Hydrologic data used in this study were provided by the Colorado Water Conservation Board (CWCB) in accordance with the Plan of Work. The original source of the discharge-frequency values was the Simons, Li & Associates, Inc. study "Cooper Slough, Boxelder Creek Master Drainageway Planning Study - 1981". The data was reviewed and concurred in by the CWCB and ourselves. The methodology for developing the discharge data was the "Storm Water Management Model" (SWMM), as revised by the Missouri River Division, U.S. Army Corps of Engineers, 1973.

Hydrologic data was based on conditions that reflected final completion of the Boxelder Creek Watershed project. A general assumption in the hydrologic data is that irrigation ditches and canals that cross the basin do not intercept flood waters from the drainages they cross. The logic behind this assumption is that the ditches and canals come into the basin full from upstream runoff and therefore have no capacity to intercept additional runoff. One exception to this assumption is the Windsor Ditch which crosses the basin at the northern boundary of the Town of Wellington. A detailed hydraulic study of this ditch showed it has the capacity to intercept about 200 cfs. from Coal Creek flows. This adjustment was made in frequency-discharge data for the portion of study area along the Coal Creek flood plain between the Windsor Ditch and its confluence with Boxelder Creek.

The frequencies concerned with in this study are the 10-year, 50-year, 100-year and 500-year events. Table 1 shows discharges at specific cross section locations. The following tabulation shows values at a few selected locations given in the previously referred to Simons, Li & Associates, Inc. Study:

Summary of Peak Flows in cfs for Boxelder Creek

Channel Location	Near X-Sec.	Drainage Area		Recurr	ence I	nternal	in Yea	ars
/		(sq mi)	2	10	25	50	100	500
U/S of Windsor Ditch	EJ	10.84	160	470	670	850	1080	1530
U/S of Colorado & Southern Railroad	DC	12.68	160	480	690	900	1140	1640
U/S of I-25 near Wellington	CA	13.86	160	480	710	920	1170	1690
D/S of I-25 near Wellington	ВІ	24 • 46	490	900	1270	1670	2140	3100
U/S of Willox Road	E	55.94	510	1140	1860	2730	3770	5820
Indian Creek	AJ	20.35	210	430	850	1350	1930	3100
Coal Creek <u>1</u> /	GH	10.60	70	230	400	600	830	1300

<sup>1/</sup> A hydraulic study of Windsor Canal indicated it would intercept flows from Coal Creek up to 200 cfs. Therefore, these discharges have been reduced by 200 cfs. from values shown in the Simons, Li & Associates, Inc. report.

The Boxelder Creek Watershed Project floodwater retarding structures reduced the 100 year peak flow at the junction of Boxelder Creek and Coal Creek from 13,300 cfs. to 1,170 cfs. At the lower end of the study, the 100 year peak flow was reduced from 11,900 cfs. to 3,770 cfs. Peak flow on Coal Creek, though the Town of Wellington, were reduced from 3,445 cfs. to 830 cfs.

#### Hydraulics

The U.S. Army Engineers HEC-2 computer program was used to perform water surface profile computations. Numerous bridges and culverts exist along the channels through the study reach. Those that created significant backwater affect were handled in the HEC-2 analysis with appropriate bridge routines. A few bridges and culverts are only minor segments of the total flood plain and therefore were considered in the Mannings retardance factor rather than in separate bridge analyses.

Cross section data, and reach length information were obtained from photogrammetric maps prepared especially for this study. Hydraulic roughness coefficients ("n" values) were determined from field inspection, and documented with photographs (in technical addendum). Following are a tabulation of these roughness coefficients for various locations along the study area.

Water surface profiles, typical cross sections and maps showing the 100 year and 500 year flood lines are shown on exhibits A, B, and Flood Hazard Area Maps. Table 1 shows computed flood elevations at specific cross sections.

Hydraulic Roughness Coefficients (n-value)

Sect	ion	. Left	Right	
From	To	Overbank	<u>Overbank</u>	Channel
Α	AJ	.040	•050	•035
AJ	BL	·050	•050	•040
BL	CI	•050	•045	•040
CI	DJ	.045	•050	.040
DJ	EJ	.040	•050	.035
EJ	FD	.045	•045	.040
BL	GH	.050	•050	.050
GH	HB	.050	.070	.050
HB	HC	.050	•050	.050
HC	HD	•050	.070	.050
HD	HF	•050	.070	.060
HF	HJ	•050	.070	.070
HJ	HL	.050	•070	.055
HL	HN	.050	.040	.040
HN	IE	.045	•045	.045

Note: Documenting photographics can be found in the technical supplement.

Significant divided flow occurs at two locations between Interstate 25 Highway and the lower study limit. This is a situation where the main channel overflows into two or more segments of flow divided by high ground. This is quite common for short distance. When it occurs for extended distances, separate water surface profiles should be made for each segment of flow. This was done at two locations along reach one. At these locations, different flood elevations may exist in a cross section accross the total flood plain. The dual profiles are included in table 1 data as well as on the flood delineation maps and plotted profiles.

#### FLOOD PLAIN MANAGEMENT

Potential flood damages to existing development and possible loss of life can be alleviated or lessened through several nonstructural and structural methods.

Nonstructural methods include: flood plain regulations, land treatment, flood warning and forecasting systems, flood insurance, flood proofing, and flood fighting and emergency evacuations.

#### Local Regulations

The need to minimize property damage due to flooding has been recognized by planners. Subdividers and developers are required to submit proposed storm drainage plans to the planning commission for approval. In the past, drainage plans have been prepared singularly or on a plat-by-plat basis. Information contained in this report will be useful in developing a master drainage plan for the study area. This report provides the outline of flood hazard areas on large scale maps specifically for this purpose.

The city may provide zoning regulations...

... "to establish, regulate, restrict, and limit such uses on or along any storm or floodwater runoff channel or basin, as such storm or floodwater runoff channel or basin has been designated and approved by the Colorado Water Conservation Board, in order to lessen or avoid the hazards to persons and damage to property resulting from the accumulation of storm or floodwaters"...

as stated in Section 30-28-111 for county governments and Section 31-23-201 for municipal governments of the Colorado Revised Statutes 1973.

#### Colorado Natural Hazard Area Regulations

In 1974, the Colorado General Assembly passed House Bill 1041, a bill "concerning land use, and providing for identification, designation, and administration of areas and activities of State interest, ..." (H.B. 1041, Title 24, Article 65.1, CRS 1973, as amended). Areas of State interest include natural hazard areas, or those areas that are "so adverse to past,

current, or forseeable construction or land use as to constitute a significant hazard to public health and safety or to property." Flood plains are natural hazard areas.

With reference to the administration of natural hazard areas, section 24-65.1-202(2)(a) of the Act provides: Flood plains shall be administered so as to minimize significant hazard to public health and safety or to property; open space activities shall be encouraged; structures shall be designed in terms of use and hazards; disposal sites and systems shall be protected from inundation by floodwaters; and activities shall be discouraged which, in time of flooding, would create significant hazards to public health and safety or to property.

The Act further provides that after promulgation of guidelines for land use in natural hazard areas ..., the natural hazard areas shall be administered by local government in a manner which is consistent with the guidelines for land use in each of the natural hazard areas.

#### Colorado Water Conservation Board Designations

Concerning the designations of flood plain, the Colorado Water Conservation Board is charged with the primary responsibility for:

- Making recommendations to local governments and the Colorado Land Use Commission.
- 2. Providing technical assistance to local governments.

The Board's power and duty is ...

..."to devise and formulate methods, means and plans for bringing about the greater utilization of the waters of the state and prevention of flood damages therefrom, and to designate and approve storm or flood-water runoff channels or basins, and to make such designations available to legislative bodies of cities and incorporated towns, to county planning commissions, and to boards of adjustment of cities, incorporated towns, and counties of this state"...

as stated in Section 37-60-106 (1) (c) of the Colorado Revised Statutes 1973.

Upon review and approval of this report, the Colorado Water Conservation Board will designate and approve as flood plain areas those areas inundated by the 100-year flood as described by the floodwater surface elevations and profiles in this report. The use of the designated flood plain areas may then be regulated by the local government.

#### Model Regulations

In the model flood plain regulations, adopted by the Colorado Water Conservation Board, the statement of purpose is to promote the public health, safety, and general welfare, and minimize flood hazards and losses by provisions designed to:

- Promote sound planning and land use, and permit only such uses within flood plains that will endanger life, health, and public safety or property in times of flooding.
- 2. Protect the public from avoidable financial expenditures for flood control projects, flood relief measures, and the repair and restoration of damaged public facilities.
- 3. Prevent avoidable interruption of business and commerce;
- 4. Minimize victimization of unwary home and land purchasers; and
- 5. Facilitate the administration of flood hazard areas by establishing requirements that must be met before use or development is permitted.

The Board's model flood plain regulations offer two options for management of the 100-year flood plain. These are the Hazard Area Concept and the Floodway Concept.

The Hazard Area concept defines the area of the flood plain in which waters of the 100-year flood attain a maximum depth greater than one and one-half feet as a high hazard area, and a depth less than this as a low hazard area.

The Board recommends that no basements should be allowed for structures located within the low hazard area and all habitable living quarters (first floors) should be constructed a minimum of one foot above the 100-year flood water surface elevations. Development is prohibited in high hazard areas.

The Floodway concept defines the channel of a stream and adjacent flood plain areas that must be kept free of development in order to safely pass the 100-year flood.

There are several methods used in floodway computations. One such theoretical method is computed on the basis of equal conveyance reduction for each side of the flood plain. Although a detailed rate study could not be made for the area included in this report, a theoretical floodway was computed for future reference. Because of the large amount of computational data, floodway information is included in Appendix II separate to this report. Data are in tabular form and include floodway widths, cross sectional flow area, and average velocities. Computations are for an increase in rise of water surface elevations in 0.5' increments from 0.0' to 1.5' above the 100-year flood.

#### Flood Insurance

The National Flood Insurance Act of 1968 (Title XIII of the Housing and Urban Development Act, P.L. 90-448) recognized the necessity for flood plain management. This Act makes federally subsidized insurance available to citizens in communities that adopt regulations controlling future developments of their flood plain. In respect to encroachment on the flood plain, the regulations require:

New residential construction or substantial improvement of existing homes must have the lowest floor level above the elevation of the 100-year flood.

Non-residential construction must meet the same standard or be flood proofed to that level.

The 1968 Act benefits owners of structures already in the flood-prone areas by providing insurance that had been unavailable through private companies. The Act created a cooperative program of insurance against flood damage by the private flood insurance industry and the federal government.

Flood insurance through the National Flood Insurance Program is available to residents of Wellington under the regular program.

The present insurance coverage is based on maps dated February 15, 1979. The amount of coverage available and the premieum rate varies considerably depending on the property location within the flood plain and the property value. All property owners shown in this study to be within areas subject to flooding should consider the purchase of flood insurance.

Additional information on the Flood Insurance Program is available from local insurance agents or brokers and the:

Federal Emergency Management Agency Division of Insurance and Mitigation Building 710 Denver Federal Center Denver, Colorado 80225

Telephone 234-6582

The National Flood Insurance Program uses the floodway concept in its' rate studies for communities participating in its' regular programs.

#### Structural Flood Control Measures

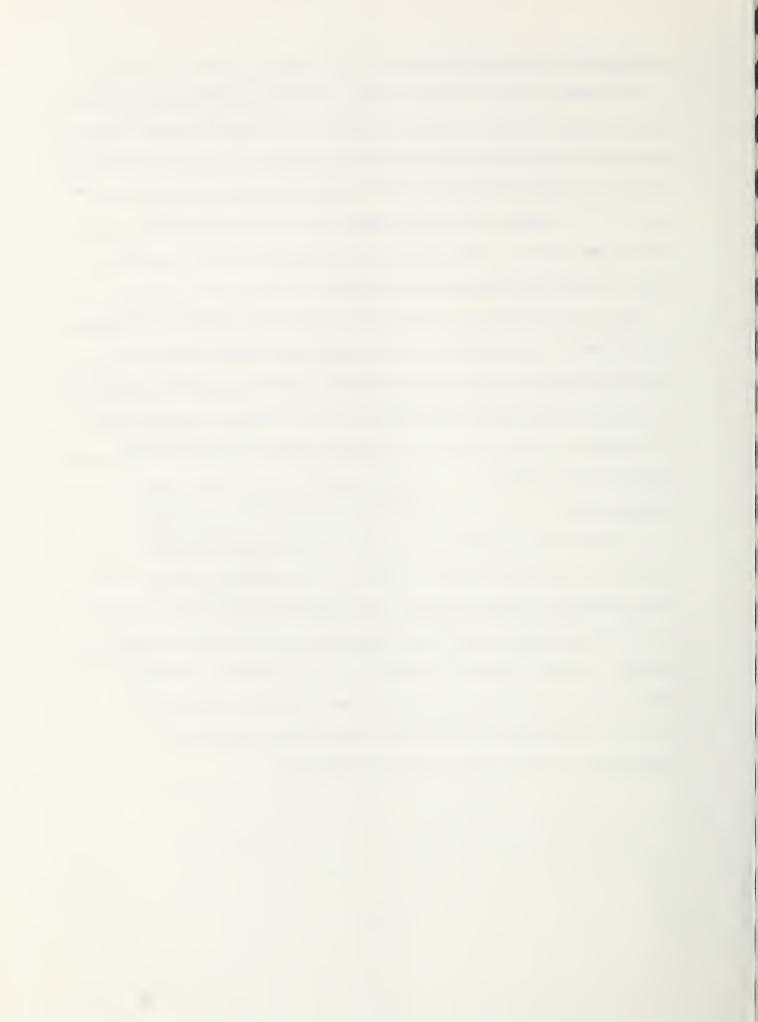
The Boxelder Creek Watershed project, completed in 1981, is located within the Boxelder Creek Watershed. Figure 1 is a map of the watershed showing the location of floodwater retarding reservoirs. This project will undoubtedly reduce flood damages along Boxelder Creek in future years. Some remaining flood damages are likely, however, because of significant uncontrolled portions of drainages below the structure.

#### Flood Warning and Flood Forecasting Systems

The National Oceanic and Atmospheric Administration (NOAA) through its National Weather Service (NWS), maintains year-around surveillance of weather and flood conditions. Daily weather forecasts are issued through the NWS and disseminated by radio and television stations. A general alert to the danger of flash flooding is one of the services provided by the National Weather Service.

#### Evacuation Plan

An "Emergency Evacuation and Operation Plan" should provide for alerting the public of potential flooding, and coordinating community and county services during an emergency. Plan implementation during the time of an emergency requires cooperation of the general public as well as local officials. This is especially important for flood fighting, evacuation, and rescue operations. Communication is extremely important during flood alerts. Warnings issued through the National Weather Service are disseminated by radio to state and local officials.



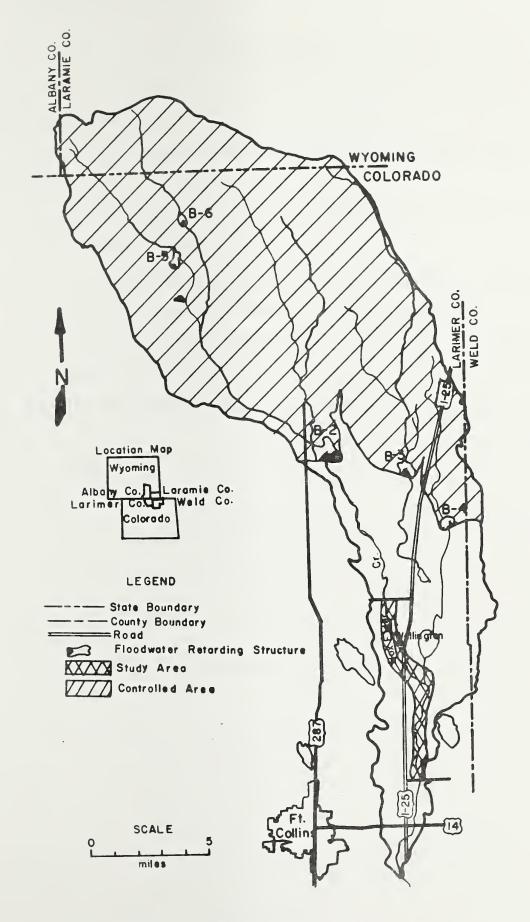
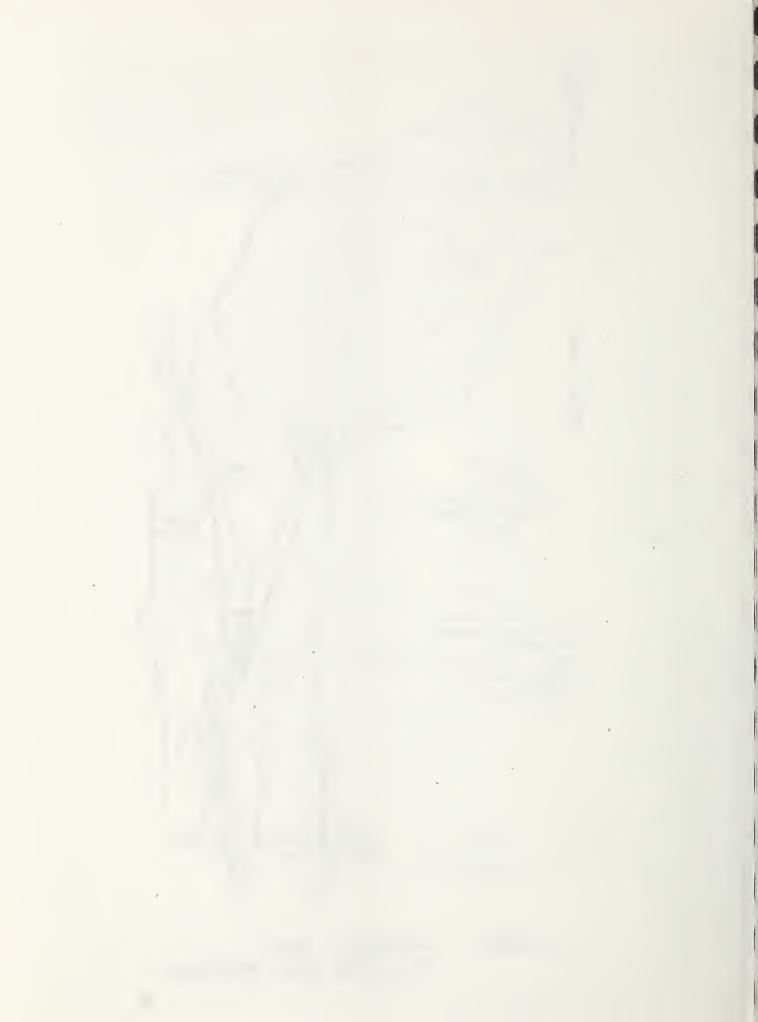


FIGURE 1. PROJECT MAP
Box Elder Creek Watershed



#### RECOMMENDATIONS

The following recommendations are included for consideration in reducing potential flood damages.

- The Town of Wellington and Larimer County should implement a flood plain management plan.
- Existing restrictions that contribute to overbank flooding should be corrected, where possible.
- 3. Owners and occupants of buildings and mobile homes within or adjacent to the delineated flood boundary should consider flood insurance.
- 4. Public information and education programs on flood hazards should be made available to the public.
- 5. Native vegetation along Boxelder Creek should be maintained.

#### GLOSSARY OF TERMS

- Channel A natural or artificial water course of perceptible extent with definite banks to confine and conduct continuously or periodically flowing water. Channel flow is that water which is flowing within the limits of the defined channel.
- Flood Water from a river, stream, water course, lake or other body of standing water, that temporarily overflows the boundaries within which it is ordinarily confined.
- Flood Crest The maximum stage or elevation reached by the waters of a flood at a given location.
- Flood Frequency A means of expressing the probability of flood occurrences as determined from a statistical analysis of representative streamflow or rainfall and runoff records. The frequency of a particular stage or discharge is usually expressed as occurring once in a specified number of years. The 10-, 25-, 50-, 100- and 500-year frequency floods have an average frequency of occurrence in the order of once in the number of years as indicated.
- 10-Year Flood A flood having an average frequency of occurrence of once in
  10 years. It has a 10 percent chance of being equaled or exceeded in
  any given year.
- 100-Year Flood A flood having an average frequency of occurrence of once in 100 years. It has a 1 percent chance of being equaled or exceeded in any given year.
- Flood Hazard Areas Areas susceptible to flood damage.
- Flood Peak The highest stage or discharge attained during a flood event;

  also referred to as peak stage or peak discharge.

- Flood Plain The relatively flat or low land area adjoining a river, stream, watercourse, lake, or other body of standing water which has been or may be covered temporarily by flood water. For administrative purposes the flood plain may be defined as the area that would be inundated by the 100-year flood.
- Perched Channel Flow A condition where the flow elevation in the outer portions of the flood plain is higher than the flow elevation in the main channel. This condition occurs when a higher secondary channel receives inflow from some location upstream and maintains a flatter slope than the main channel.
- Reach A hydraulic engineering term used to describe longitudinal segments of a stream or river.
- Runoff That part of precipitation, as well as any other flow contributions, which appears in surface streams of either perennial or intermittent form.
- <u>Stream</u> Any natural channel or depression through which water flows whether continuously, intermittently, or periodically, including modification of the natural channel or depression.
- Structure Anything constructed or erected, the use of which requires a more or less permanent location on or in the ground. Includes but is not limited to bridges, buildings, canals, dams, ditches, diversions, irrigation systems, pumps, pipelines, railroads, roads, sewage disposal systems, underground conduits, water supply systems and wells.
- Typical Valley Cross Section An engineering drawing of a vertical section of a stream channel and adjoining landscape as viewed in a downstream direction. The drawing represents a specified location within a designated stream reach.

- <u>Water Surface Profile</u> (This term is synonymous with Flood Profile) a graph showing the relationship of the water surface elevation of a flood event to location along a stream or river.
- <u>Watersheds</u> A drainage basin or area which collects runoff and transmits it usually by means of streams and tributaries to the outlet of the basin.

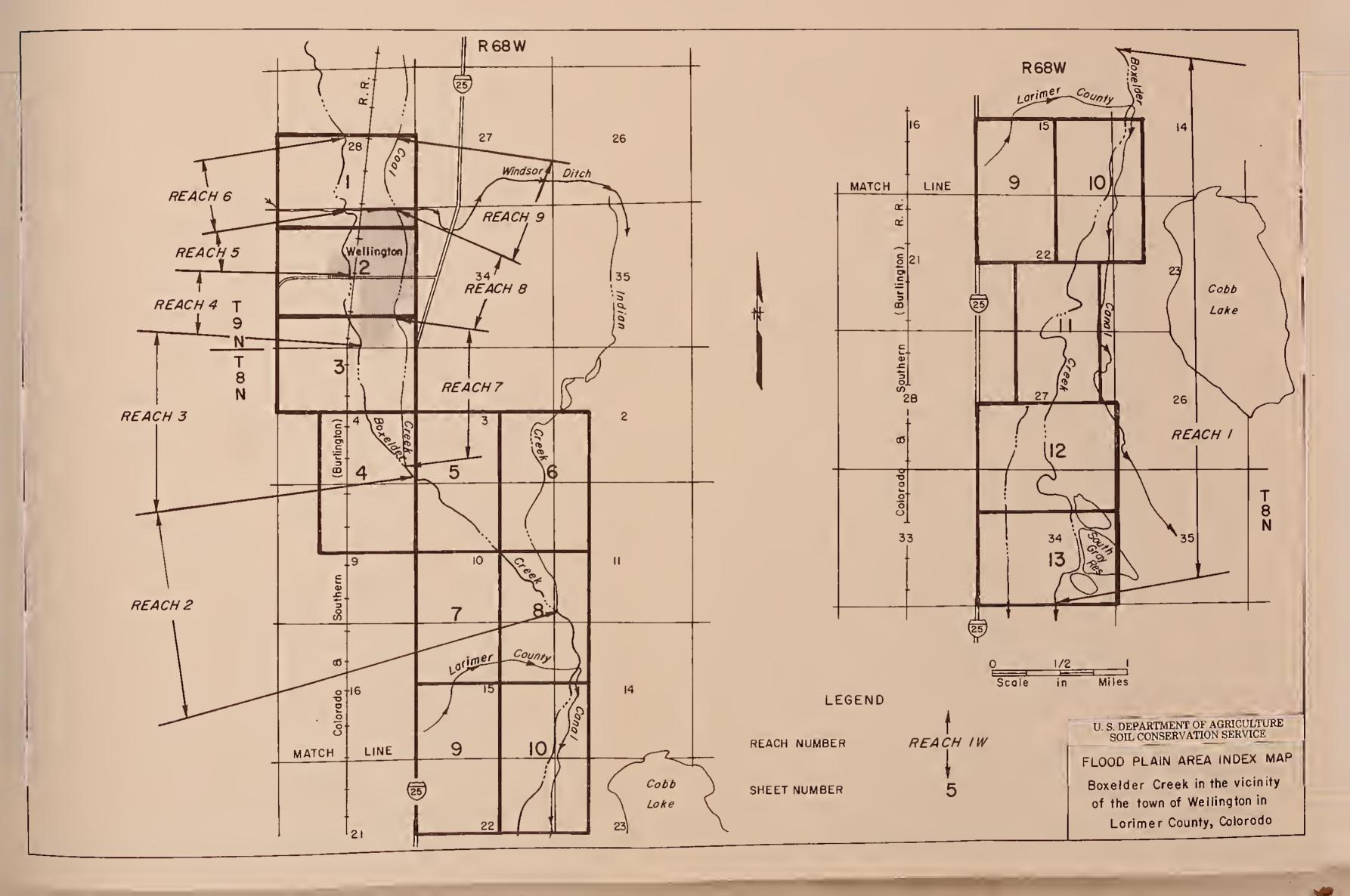
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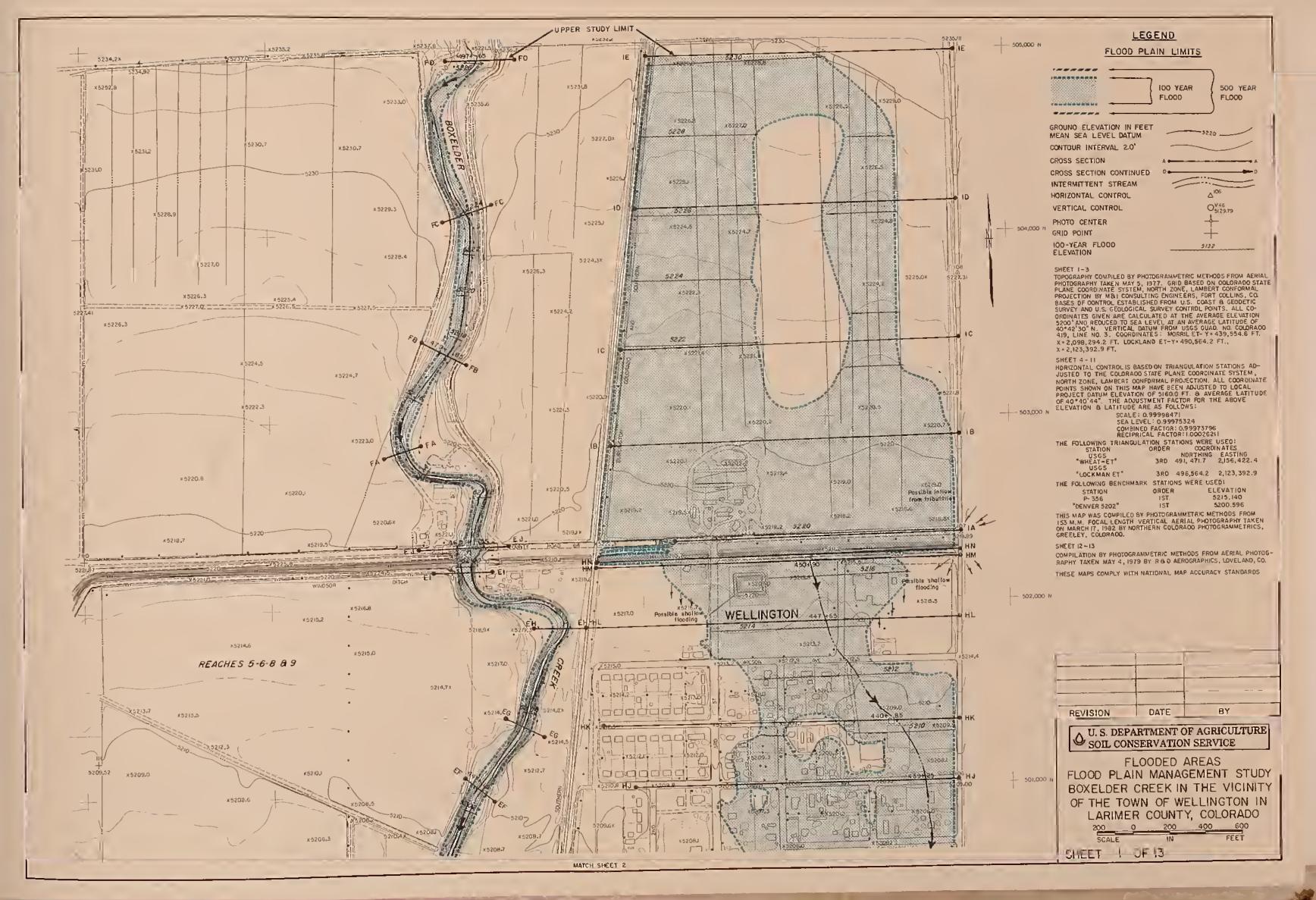
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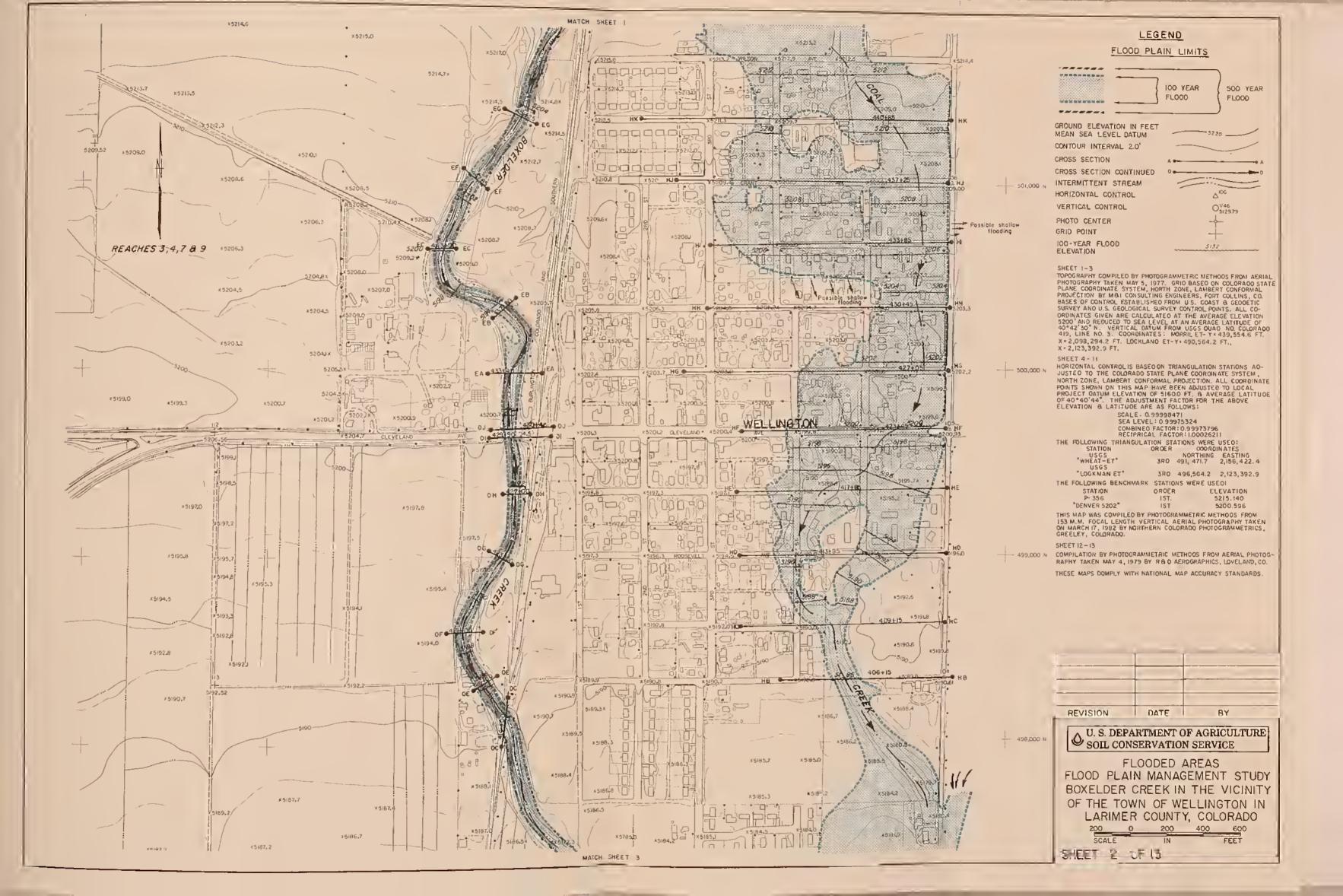
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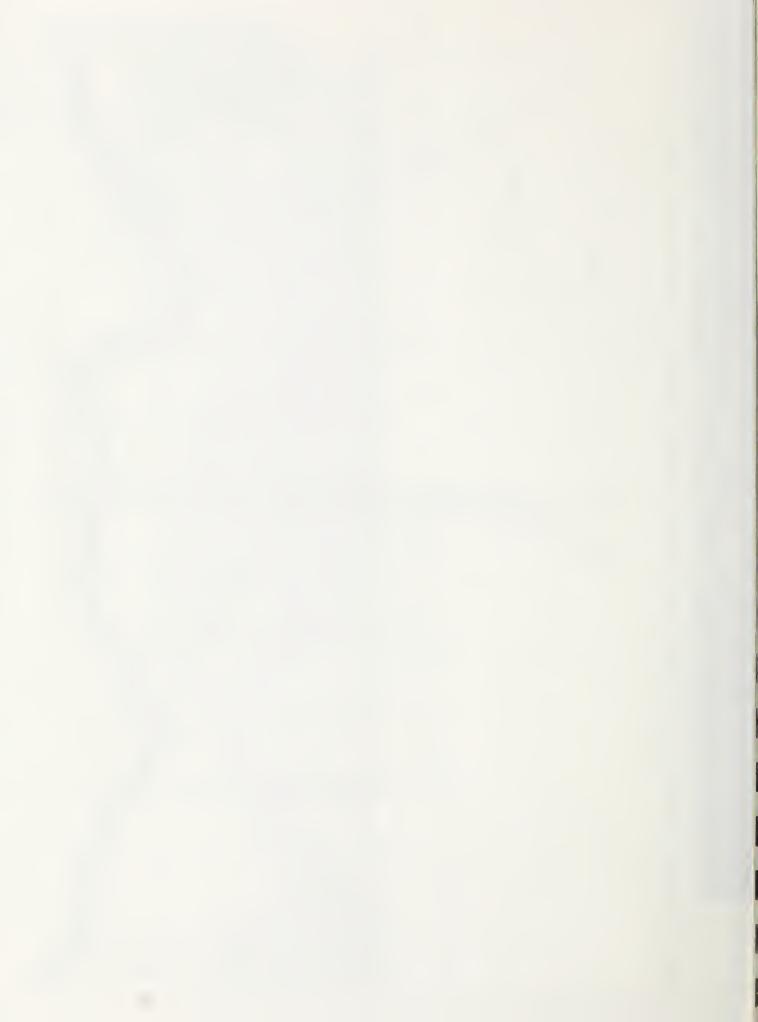


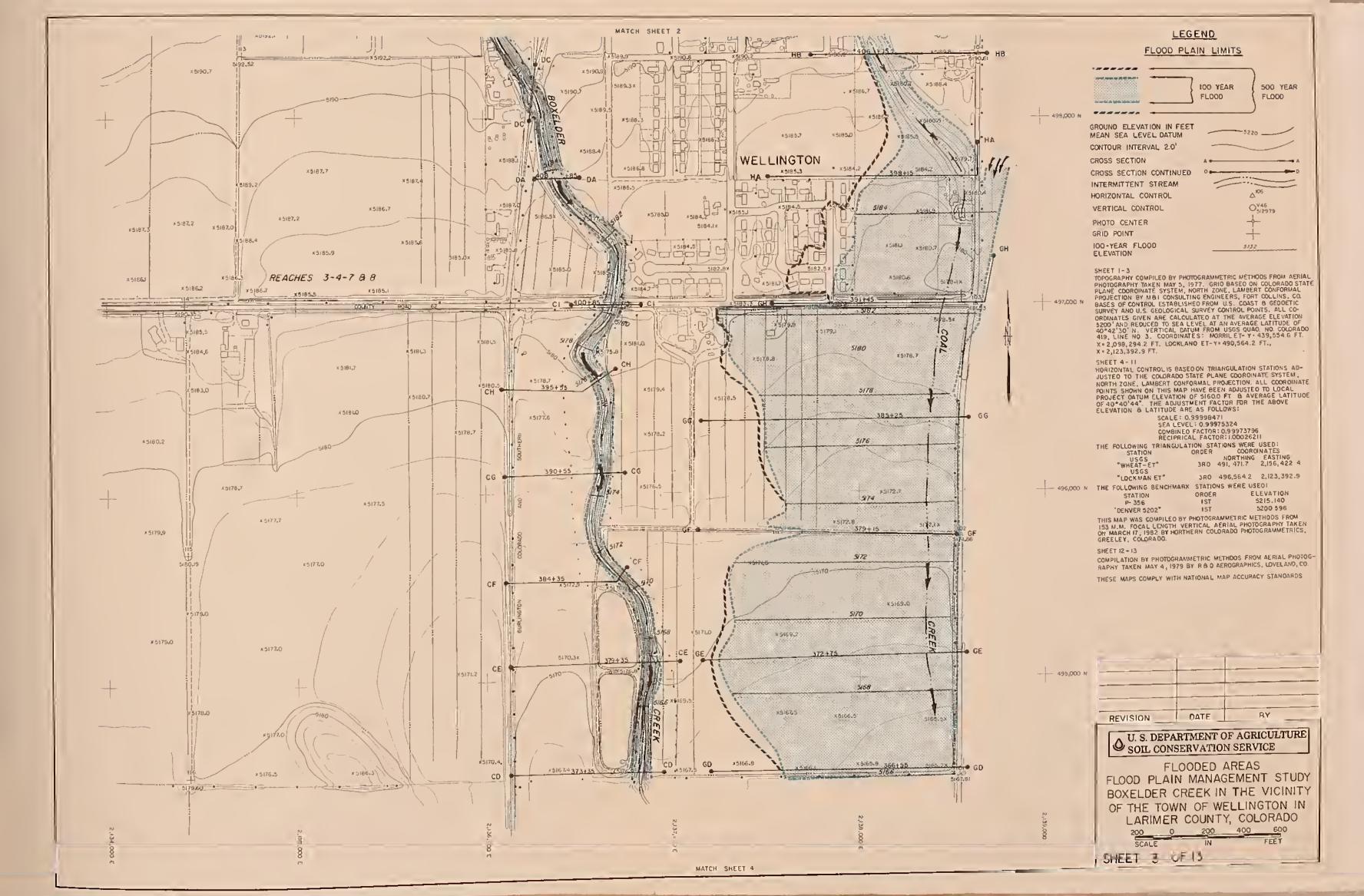




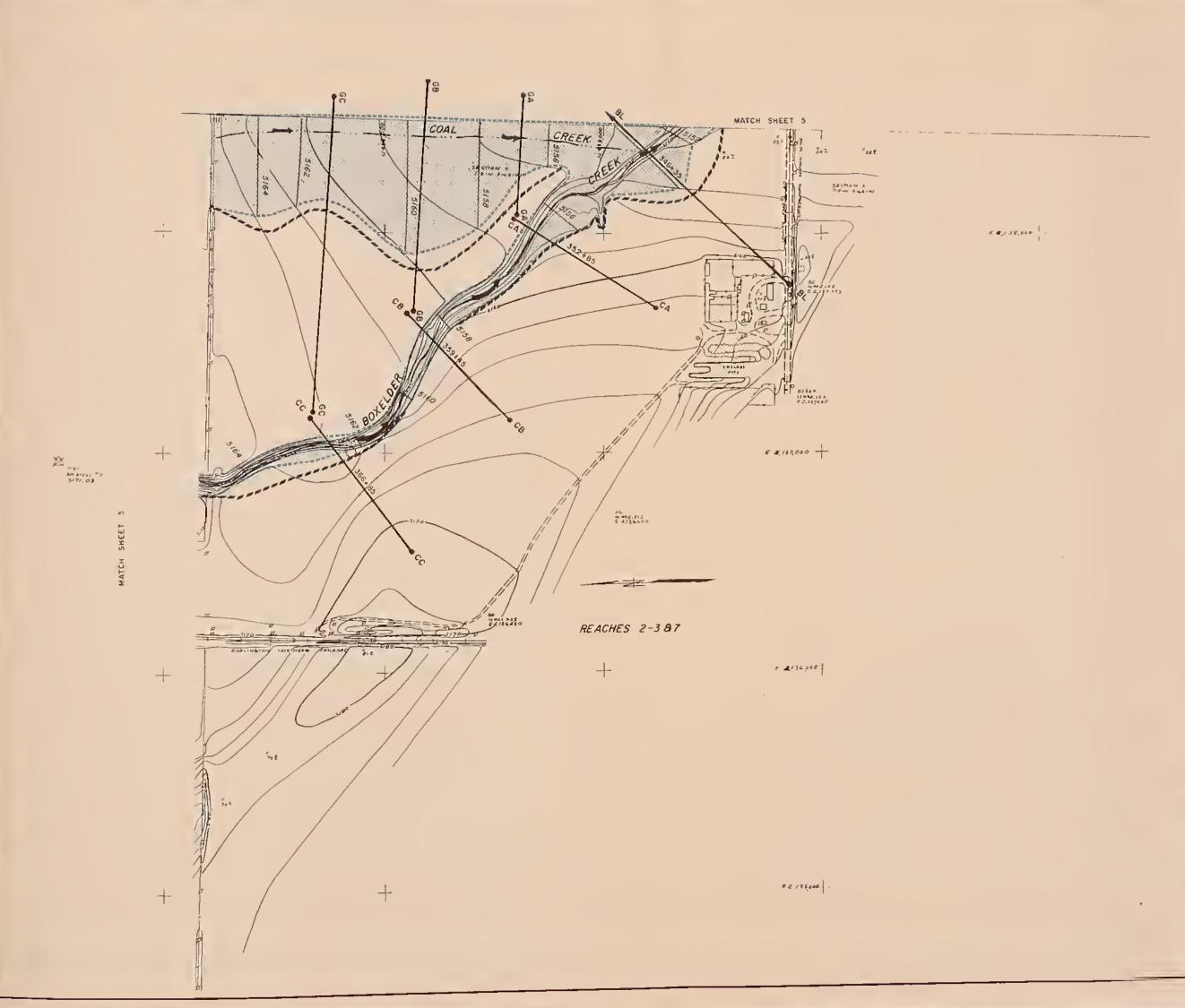






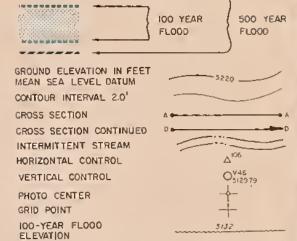






### LEGEND

### FLOOD PLAIN LIMITS



SHEET 1-3
TOPOGRAPHY COMPILED BY PHOTOGRAMMETRIC METHODS FROM AERIAL PHOTOGRAPHY TAKEN MAY 5, 1977. GRID BASED ON COLDRADD STATE PLANE COORDINATE SYSTEM, NORTH ZONE, LAMBERT CONFORMAL PROJECTION BY MB! CONSULTING ENGINEERS, FORT COLLINS, CO. BASES OF CONTROL ESTABLISHED FROM U.S. COAST & GEODETIC SURVEY AND U.S. GEOLOGICAL SURVEY CONTROL POINTS, ALL CO-DRDINATES GIVEN ARE CALCULATED AT THE AVERAGE ELEVATION 5200' AND REDUCED TO SEA LEVEL AT AN AVERAGE LATITUDE OF 40°42'30" N. VERTICAL DATION FROM USGS GUAD, NO. COLORADO 419, LINE ND 3. COORDINATES: MORRIL ET- Y = 439,554 G FT. X = 2,123,392.9 FT.

SHEET 4-11 SHEET 4-II
HORIZONTAL CONTROL IS BASED ON TRIANGULATION STATIONS ADJUSTED TO THE COLORAGO STATE PLANE COORDINATE SYSTEM,
NORTH ZDNE, LAMBERT CONFORMAL PROJECTION, ALL COORDINATE
POINTS SHOWN ON THIS MAP HAVE BEEN ADJUSTED TO LOCAL
PROJECT DATUM ELEVATION OF 5160, FT, 8 AVERAGE LATITUDE
DF 40'44". THE ADJUSTMENT FACTOR FOR THE ABOVE
ELEVATION & LATITUDE ARE AS FOLLOWS:

SCALE: 0.9999847!
SEA LEVEL: 0.99975324
COMBINED FACTOR: 0.99973796
RECIPRICAL FACTOR: 1.000262!!
THE FOLLOWING TRIANGULATION STATIONS WERE USED:

THE FOLLOWING TRIANGULATION STATIONS WERE USED:
STATION ORDER COORDINATES
USGS NORTHING EASTING
"WHEAT-ET" 3RD 491, 471.7 2,156,422.4 USGS
"WHEAT~ET"
USGS
"LOCKMAN ET" 3RD 496,564.2 2,123,392.9 THE FOLLOWING BENCHMARK STATIONS WERE USED!

STATION P- 356 DRDER IST. IST ELEVATION 5215,140 "0ENVER 5202"

THIS MAP WAS COMPILED BY PHOTOGRAMMETRIC METHODS FROM 153 M.M. FOCAL LENGTH VERTICAL AERIAL PHOTOGRAMMETRICS ON MARCH IT, 1982 BY NORTHERN COLORADO PHOTOGRAMMETRICS, GREELEY, COLORADO.

COMPILATION BY PHOTOGRAMMETRIC METHODS FROM AERIAL PHOTOGRAPHY TAKEN MAY 4, 1979 BY R 8 D AEROGRAPHICS, LOVELAND, CO

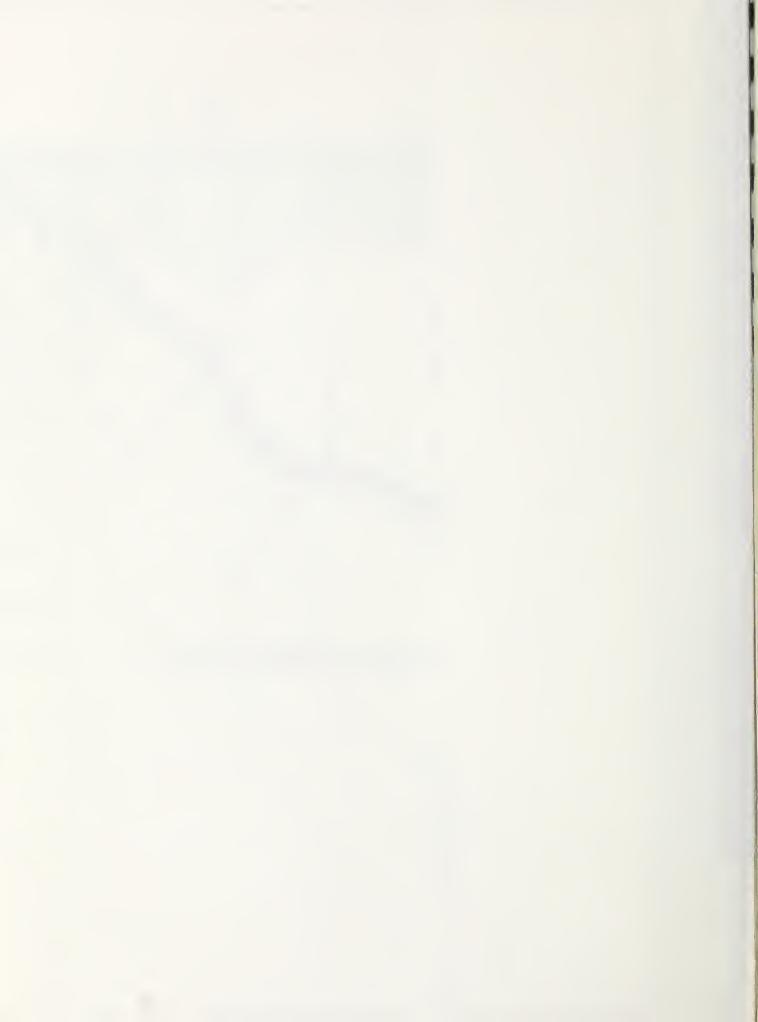
THESE MAPS COMPLY WITH NATIONAL MAP ACCURACY STANDARDS.



# U. S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE

FLOODED AREAS FLOOD PLAIN MANAGEMENT STUDY BOXELDER CREEK IN THE VICINITY OF THE TOWN OF WELLINGTON IN LARIMER COUNTY, COLORADO





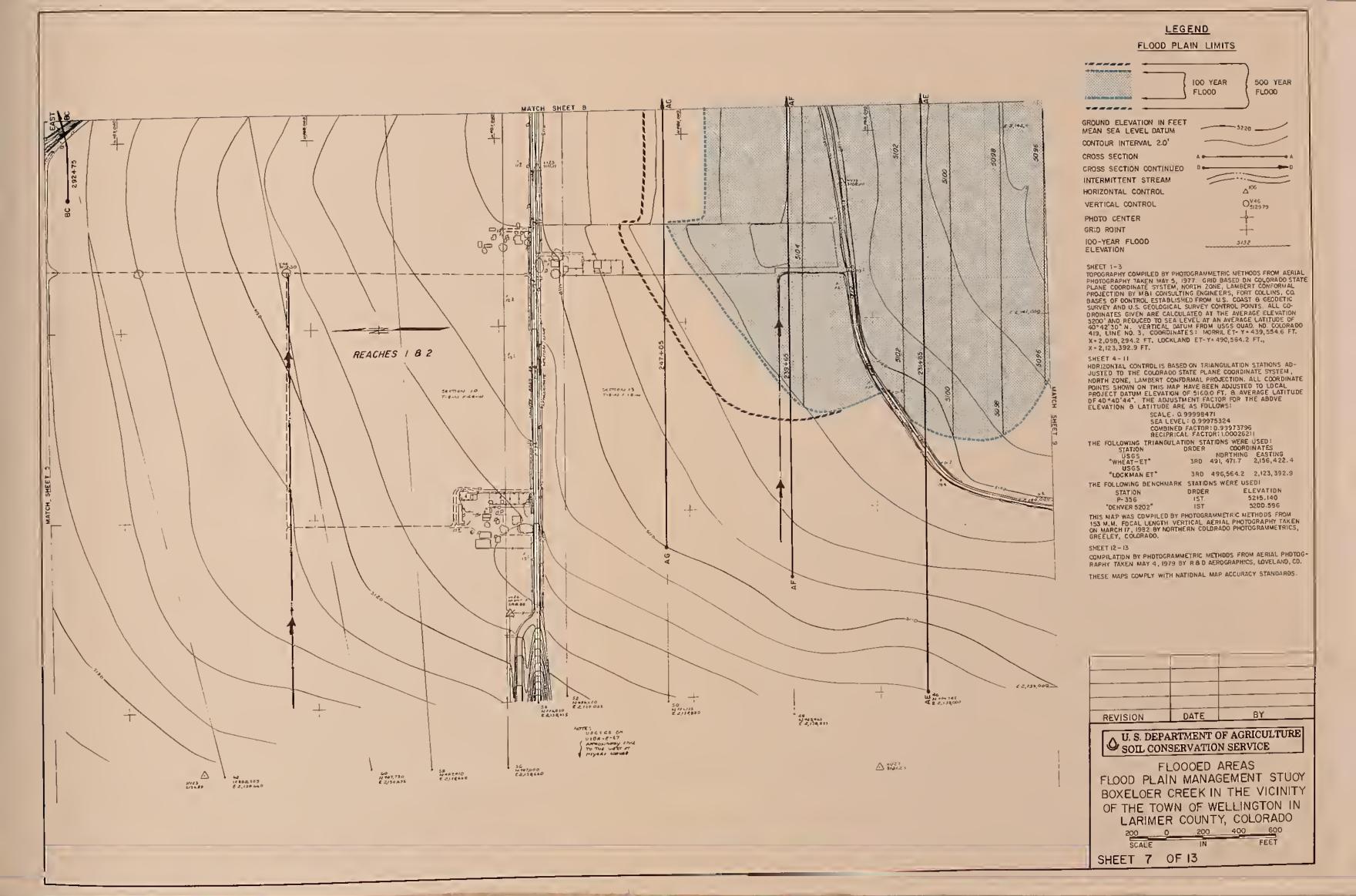
## FLOOD PLAIN LIMITS 100 YEAR 500 YEAR FL000 FL000 -----2 2 4 2 00 S GROUND ELEVATION IN FEET MEAN SEA LEVEL DATUM CONTOUR INTERVAL 2.0' flooding 0. CROSS SECTION CROSS SECTION CONTINUED INTERMITTENT STREAM HORIZONTAL CONTROL O512979 VERTICAL CONTROL PHOTO CENTER GRID POINT 100-YEAR FLOOD ELEVATION SHEET 1-3 TOPOGRAPHY CDMPILED BY PHOTOGRAMMETRIC METHODS FROM AERIAL PHOTOGRAPHY TAKEN MAY 5, 1977. GRID BASED DH COLDRADO STATE PLANE COORDIHATE SYSTEM, NORTH ZONE, LAMBERT CONFORMAL PROJECTION BY MBI CONSULTING ENGINEERS, FDRT COULINS, CO. BASES DF COHTROL ESTABLISHED FROM U.S. COAST & GEODETIC SURVEY AND U.S. GEOLOGICAL SURVEY CONTROL POHTS, ALL CO-DRDINATES GIVEN ARE CALCULATED AT THE AVERAGE ELEVATION 5200' AND REQUICED TO SEA LEVEL AT AN AVERAGE LATITUDE OF 40°42'30' N. VERTICAL DATUM FROM USOS DUAD. NOL COLORADO 419, LINE HD. 3. COORDINATES: MORRIL ET- Y= 439,554.6 FT X=2,099,294.2 FT, LOCKLAHD ET-Y=490,564.2 FT., SHEET 4 - 11 MDRIZDNTAL CONTROL IS BASED ON TRIANGULATION STATIONS ADJUSTED TD THE COLOR ADO STATE PLANE COORDINATE SYSTEM, NORTH ZONE, LAMBERT CONFORMAL PROJECTION. ALL COORDINATE POINTS SHOWN DN THIS MAP HAVE BEEN ADJUSTED TO LDCAL PROJECT DATUM ELEVATION DF 5160,D FT. 8 AVERAGE LATITIDE DF 4D\*40\*44\*. THE ADJUSTMENT FACTOR FOR THE ABDVE ELEVATION B LATITUDE ARE AS FOLLOWS: SCALE: 0.9999847! SCALE: 0.9999847! SCALE: 0.9999847! SCALE: 0.9999847! SCALE: 0.99975324 CDMBINED FACTOR: 0.000262!! THE FOLLOWING TRIANGULATION STATIONS WERE USED: STATION DRDER COORDINATES TOORDINATES WHEAT-ET\* 3RD 491, 474.7 2,136, 422.4 USGS "LOCKMAN ET" 3RD 496,564.2 2,123,392.9 SHEET 4-11 Possible shallow REACH 2 BO flooding 3RD 496,564.2 2,423,392.9 THE FOLLOWING BENCHMARK STATIONS WERE USED! STATION DRDER ELEVA ELEVATION 5215.14D STATION P- 356 IST IST THIS MAP WAS COMPILED BY PHOTOGRAMMETRIC METHODS FROM 153 M.M. FOCAL LEHGTH VERTICAL AERIAL PHOTOGRAPHY TAKEN ON MARCH IT, 1982 BY NORTHERN COLD RADO PHOTOGRAMMETRICS, GREELEY, COLORADO. "DENVER 5202" 52DD.596 COMPILATION BY PHOTOGRAMMETRIC METHODS FROM AERIAL PHOTOG-RAPNY TAKEN MAY 4, 1979 BY R B D AEROGRAPHICS, LOVELAND, CO. BOXEL THESE MAPS COMPLY WITH NATIONAL MAP ACCURACY STANDARDS. 573 DATE REVISION U. S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE # 485,760 # 485,760 INTERSTATE HIGHWAY 25 FLOODED AREAS FLOOD PLAIN MANAGEMENT STUDY BOXELDER CREEK IN THE VICINITY OF THE TOWN OF WELLINGTON IN MATCH SHEET 4 LARIMER COUNTY, COLORADO SHEET 5 OF 13

LEGEND

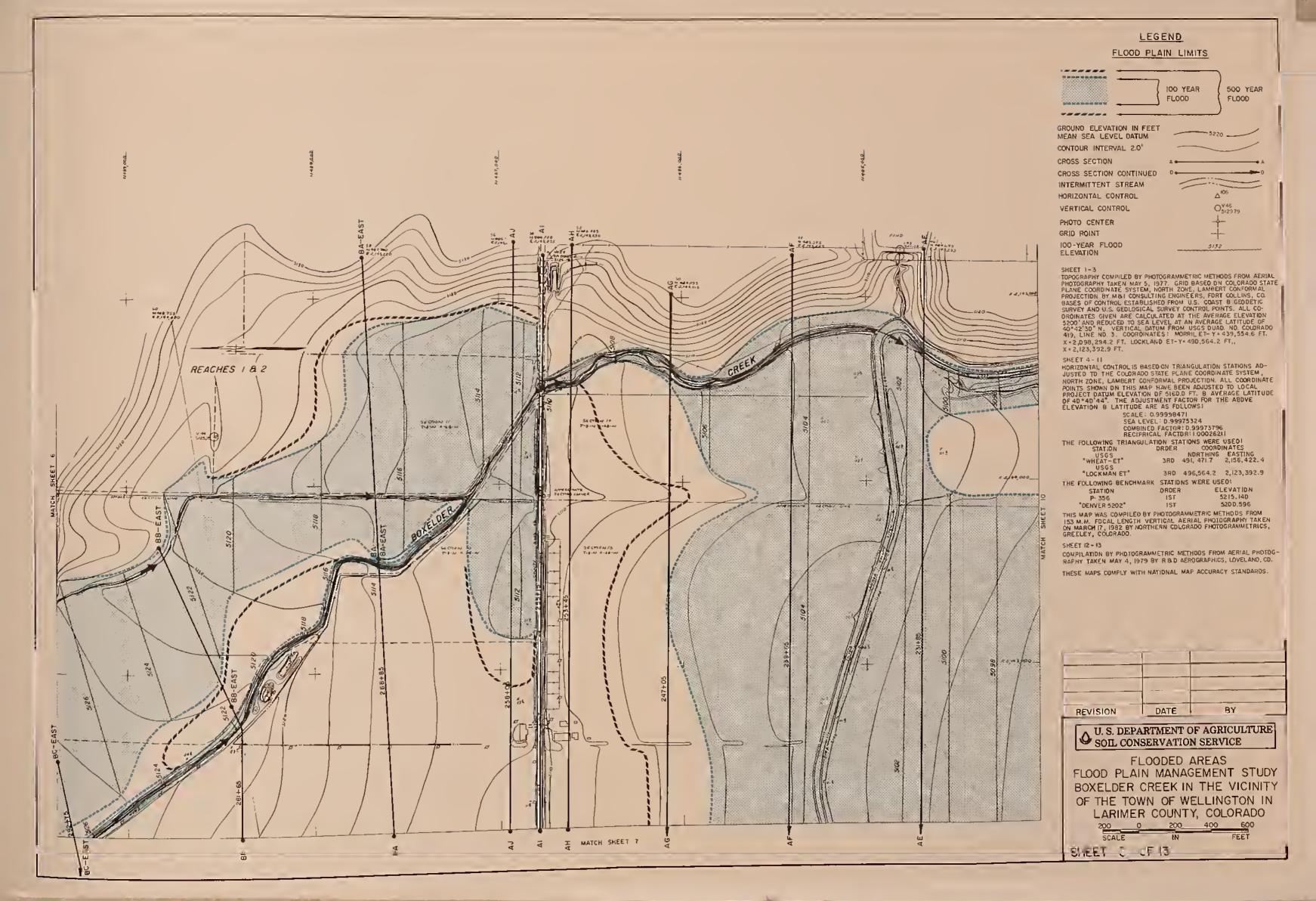


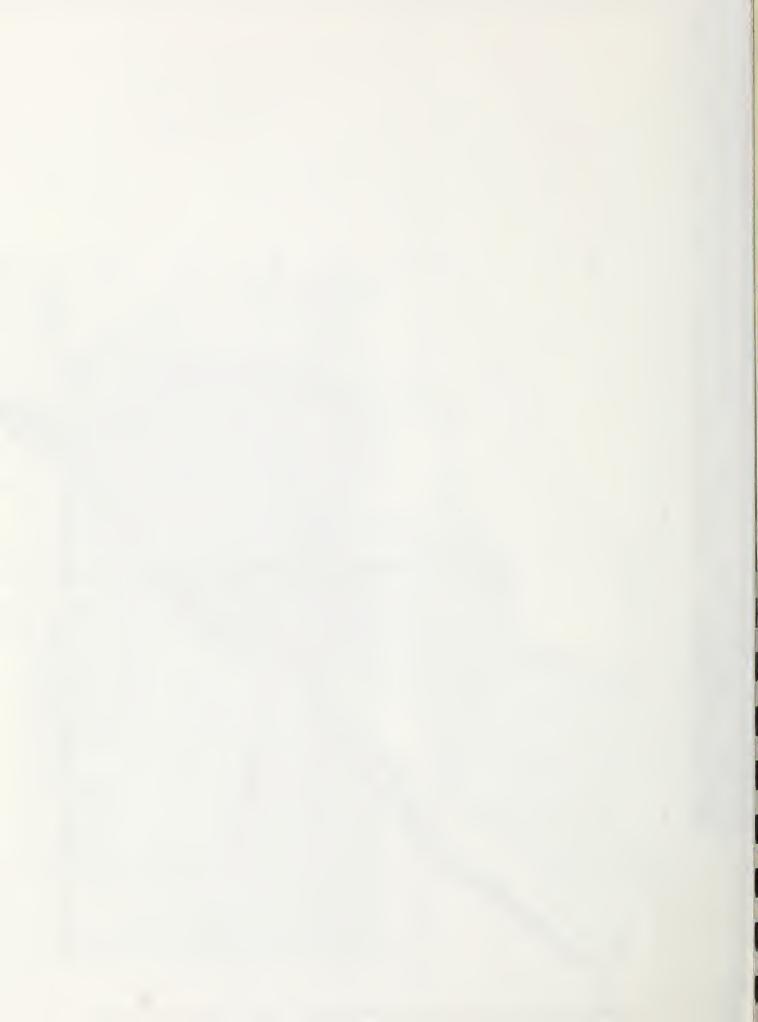
## LEGEND FLOOD PLAIN LIMITS 100 YEAR 500 YEAR FLOOD FL000 GROUND ELEVATION IN FEET MEAN SEA LEVEL DATUM CONTOUR INTERVAL 2.0' CROSS SECTION CROSS SECTION CONTINUED INTERMITTENT STREAM HORIZONTAL CONTROL VERTICAL CONTROL PHOTO CENTER GRID ROINT 100-YEAR FLOOD ELEVATION SHEET 1-3 TOPOGRAPHY COMPILED BT PHOTOGRAMMETRIC METHODS FROM AERIAL PHOTOGRAPHY TAKEN MAY \$, 1977. GRID BASED ON COLORAGO STATE PLANE COORDINATE SYSTEM, NORTH ZONE, LAMBERT CONFORMAL PROJECTION BY MB I CONSULTING ENGINEERS, FORT COLLINS, CO. BASES OF CONTROL ESTABLISHED FROM U.S. COAST B GEDDETIC SURVEY AND U.S. GEOLOGICAL SURVEY CONTROL POINTS. ALL CODRIGINATES GIVEN ARE CALCULATED AT THE AVERAGE ELEVATION 5200' AND REQUICED TO SEA LEVEL AT AN AVERAGE LATITUDE DF 40\*42'30' N. VERTICAL DATUM FROM USGS DUAD. NO. COLORAGO 419, LINE NO. 3. COORDINATES: MORRIL ET-Y\*439,554,6 FT, X\*2,098,294.2 FT. LOCKLAND ET-Y\*49D,564.2 FT., X\*2,123.392.9 FT. SHEET 4-11 HORIZONTAL CONTROL IS BASED ON TRIANGULATION STATIONS ADJUSTED TO THE COLORAGO STATE PLANE COORDINATE SYSTEM, NORTH ZONE, LAMBERT CONFORMAL PROJECTION. ALL COORDINATE POINTS SHOWN DN THIS MAP HAVE BEEN ADJUSTED TO LDCAL PROJECT DATUM ELEVATION DF SIGDD FT. B AVERAGE LATITUDE DF 4D\*40\*44\*. THE ADJUSTMENT FACTOR FOR THE ABOVE ELEVATION B LATITUDE ARE AS FOLLOWS: SCALE: 0.9999847! SCALE: 0.9999847! SCALE: 0.9999847! SCALE: 0.99973724 COMBINED FACTOR: 0.99973726 RECIPRICAL FACTOR: 1.000262! THE FOLLOWING TRIANGULATION STATIONS WERE USED: STATION USGS "WHEAT-ET" 3RO 491, 471.7 2,156,422.4 SHEET 4-11 REACH 2 USGS "LOCKMAN ET" 3RD 496,564.2 2,123,392.9 THE FOLLOWING BENCHMARK STATIONS WERE USED! ORDER SYATION P- 356 IST. IST 5200.596 "DENVER 5202" THIS MAP WAS COMPILED BY PHOTOGRAMMETRIC METHODS FROM 153 M.M. FDCAL LENGTH VERTICAL AERIAL PHOTOGRAPHY TAKEN ON MARCH 17, 1982 BY NORTHERN COLDRADO PHOTOGRAMMETRICS, GREELEY, COLDRADO. SHEET 12 - 13 COMPILATION BY PHOTOGRAMMETRIC METHODS FROM AERIAL PHOTOGRAPHY TAXEN MAY 4, 1979 BY R & D AEROGRAPHICS, LOVELAND, CD. THESE MAPS COMPLY WITH NATIONAL MAP ACCURACY STANDARDS. DATE U. S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE FLOODED AREAS FLOOD PLAIN MANAGEMENT STUDY 265 BOXELDER CREEK IN THE VICINITY OF THE TOWN OF WELLINGTON IN MATCH SHEET 5 LARIMER COUNTY, COLORADO CHEET 8 OF 13

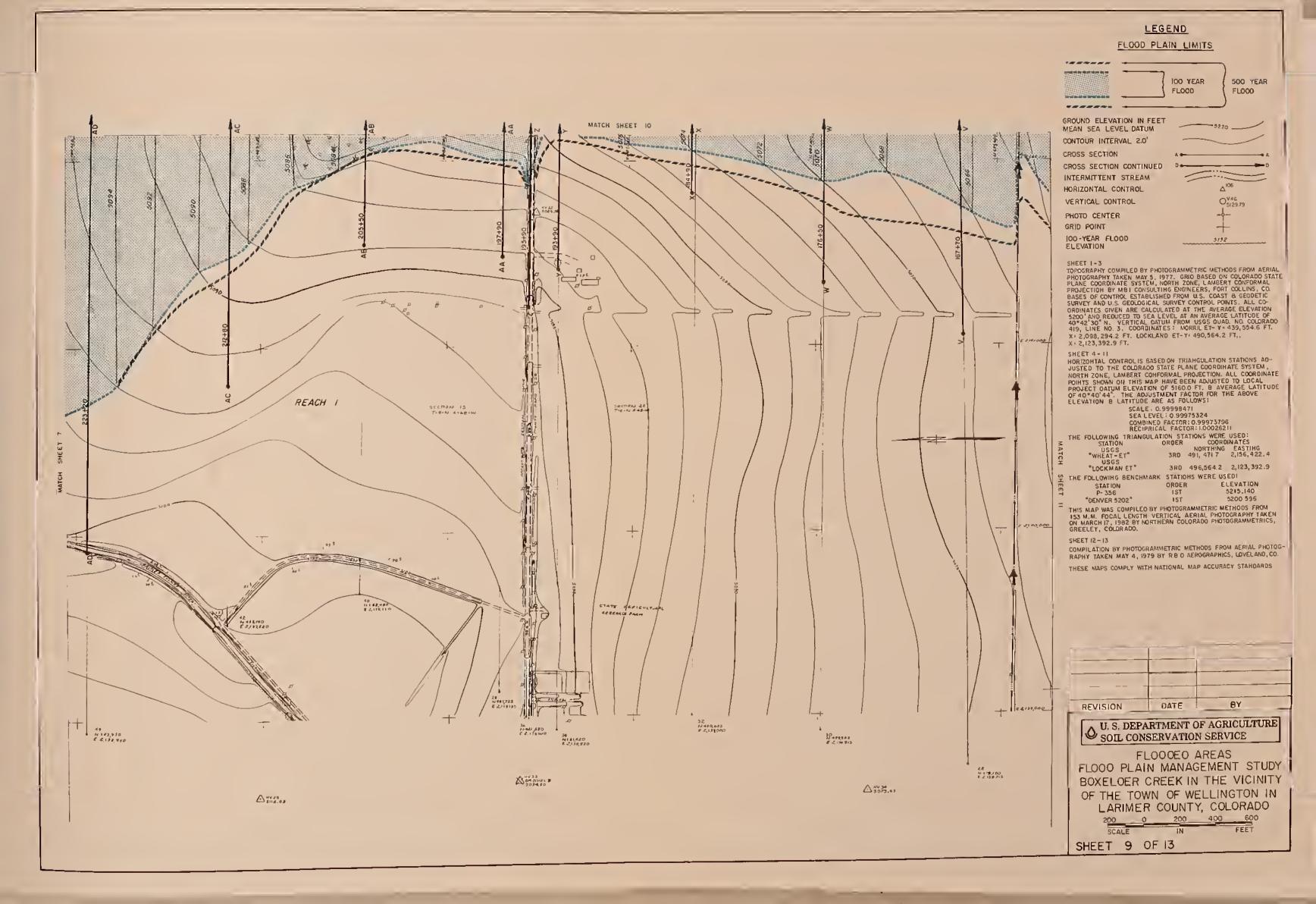




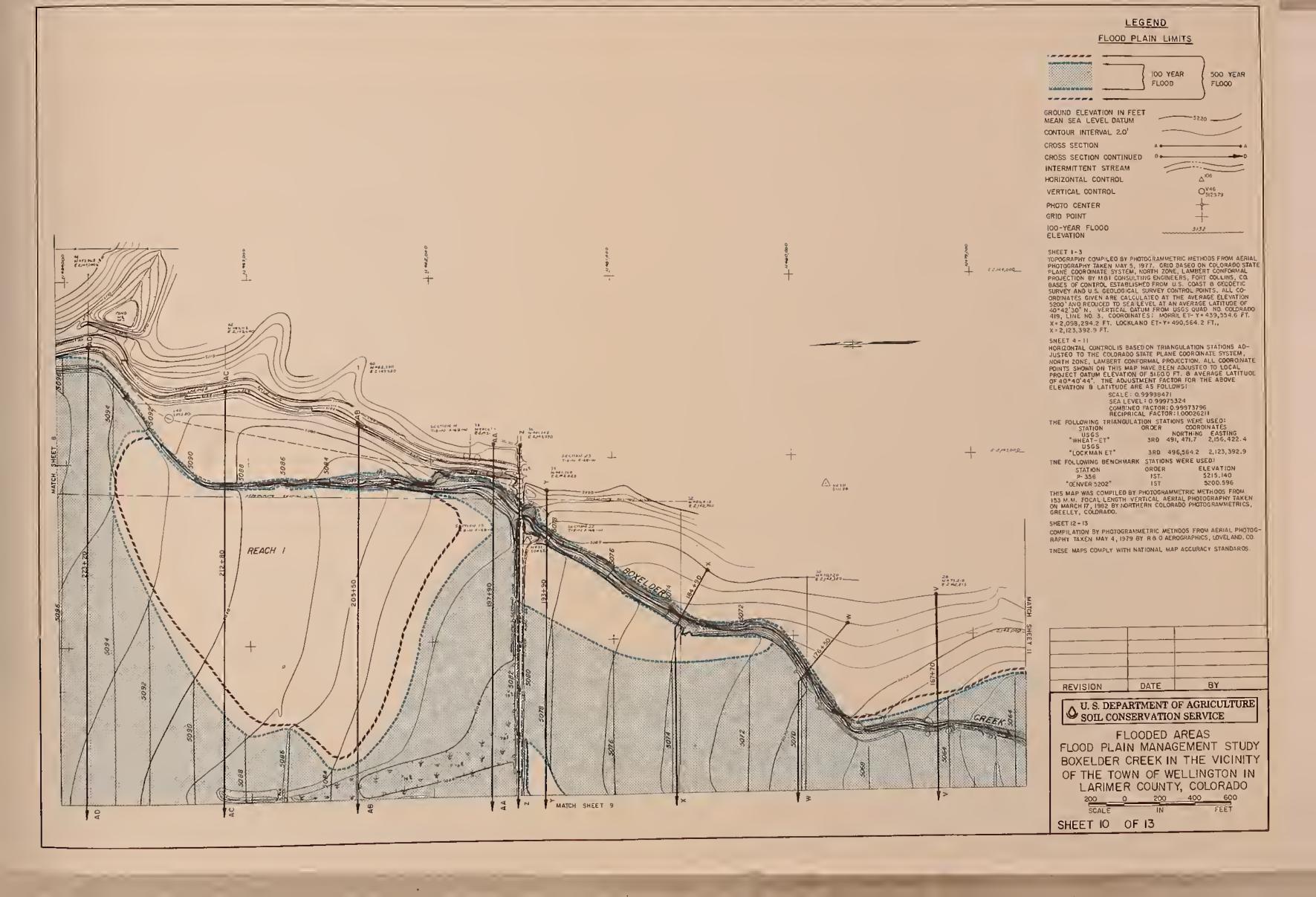


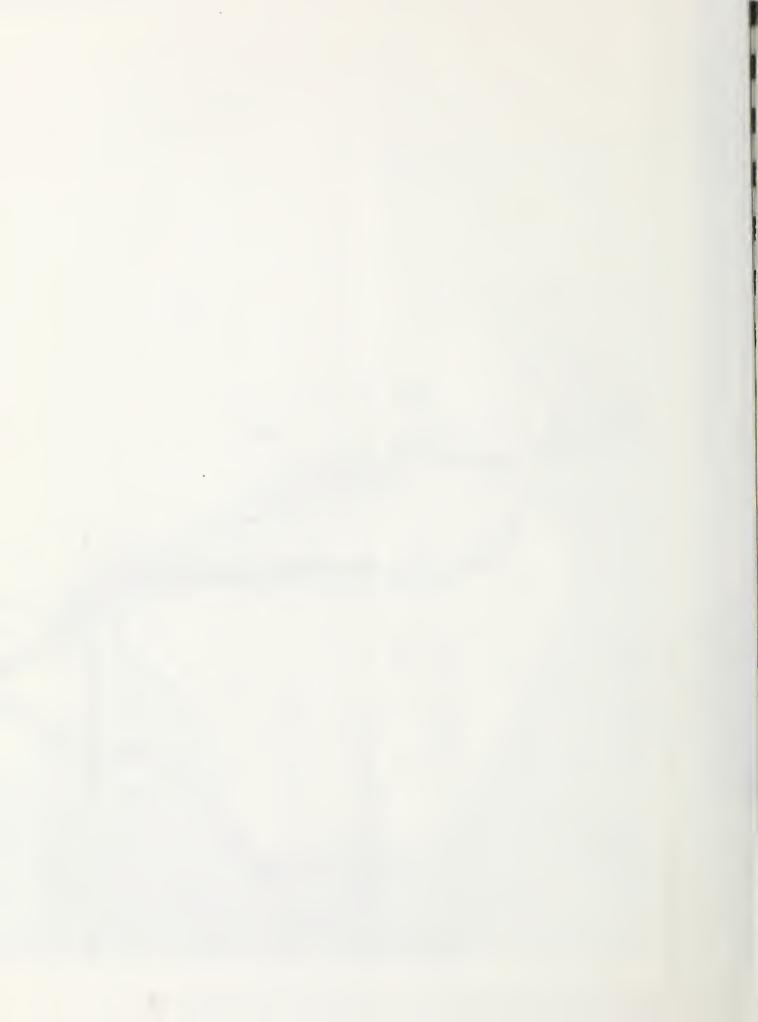


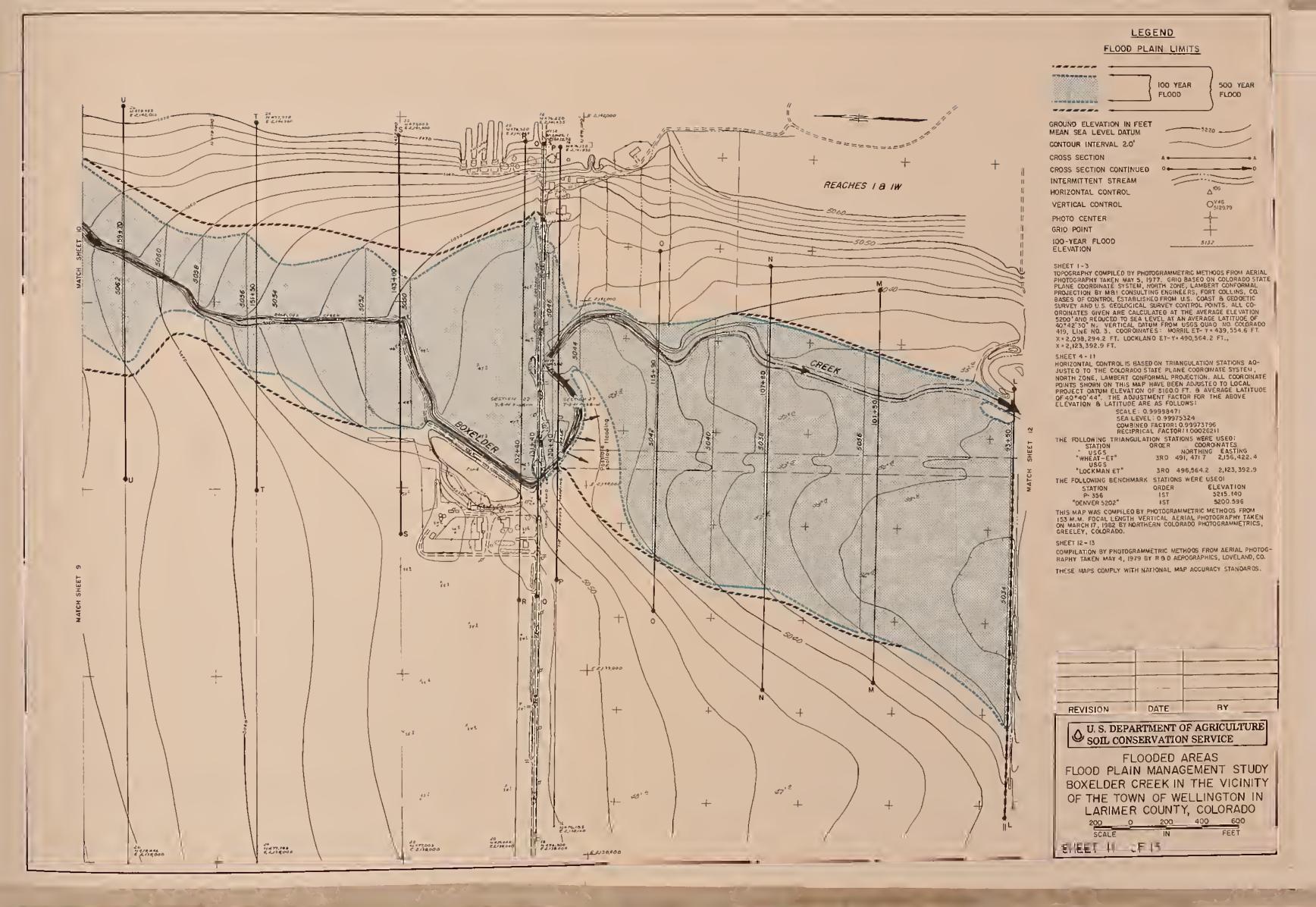




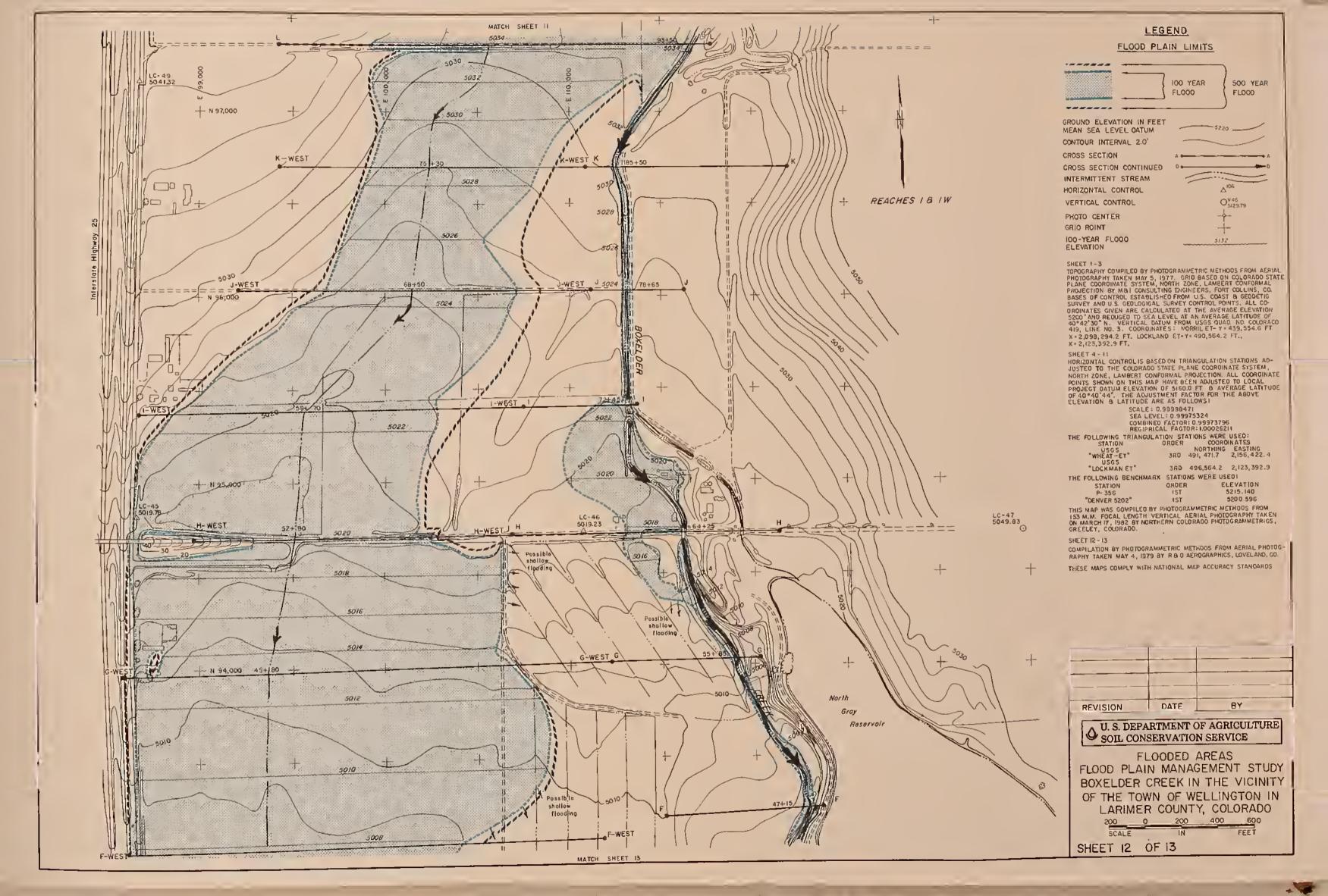




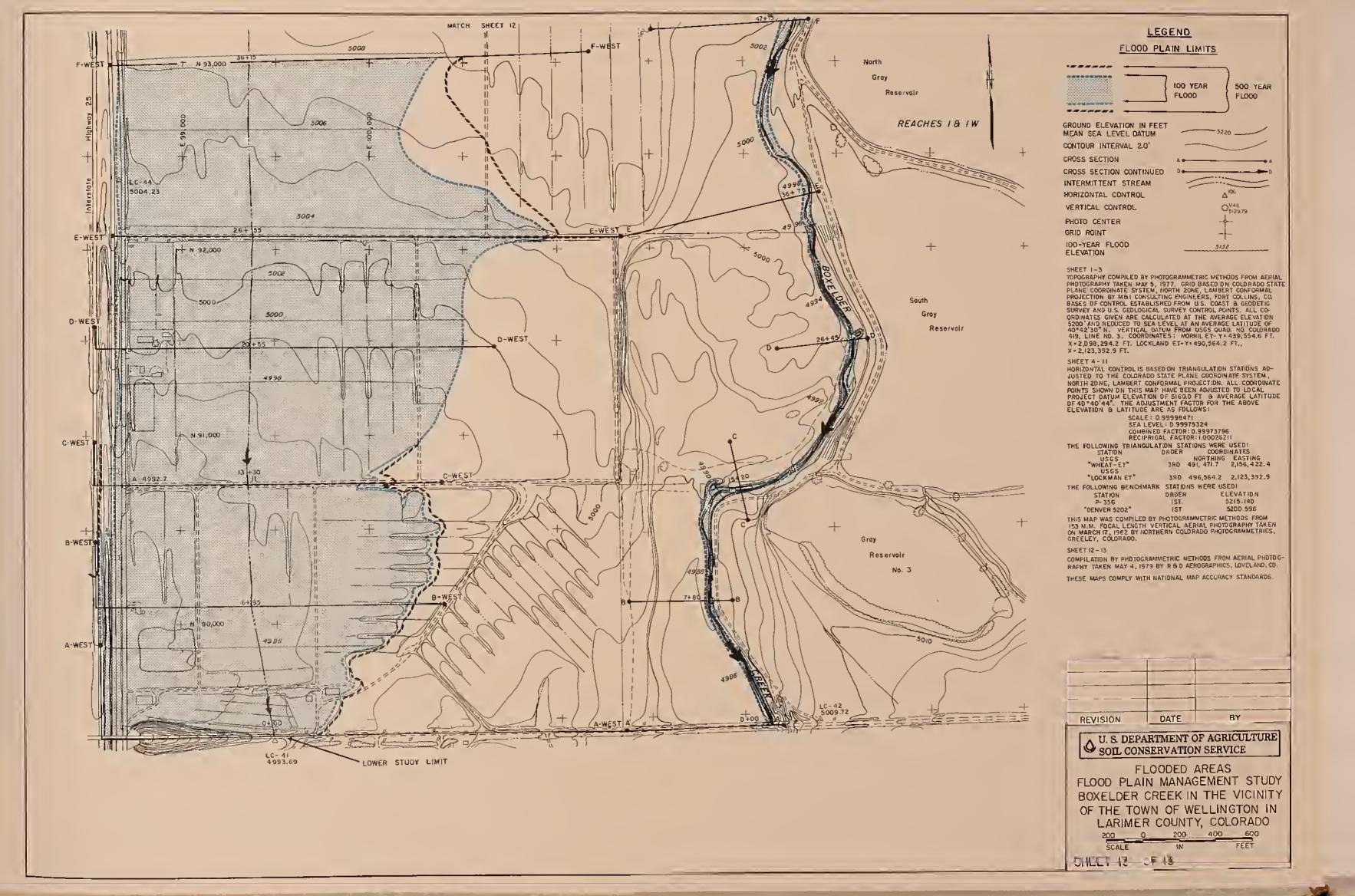


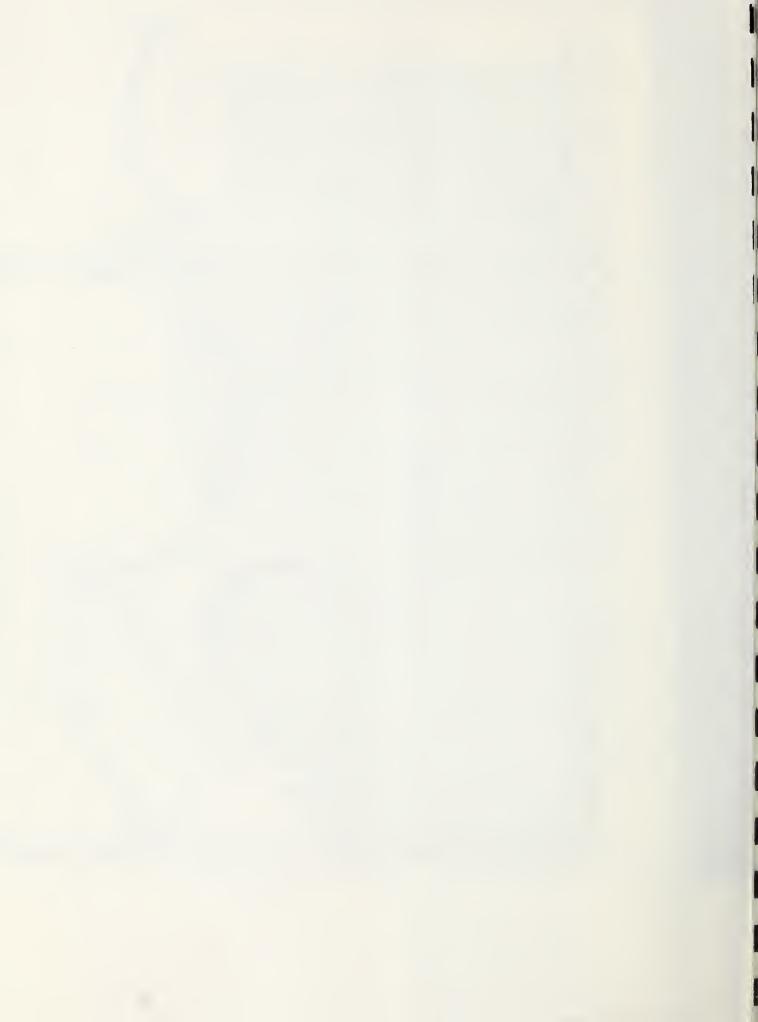


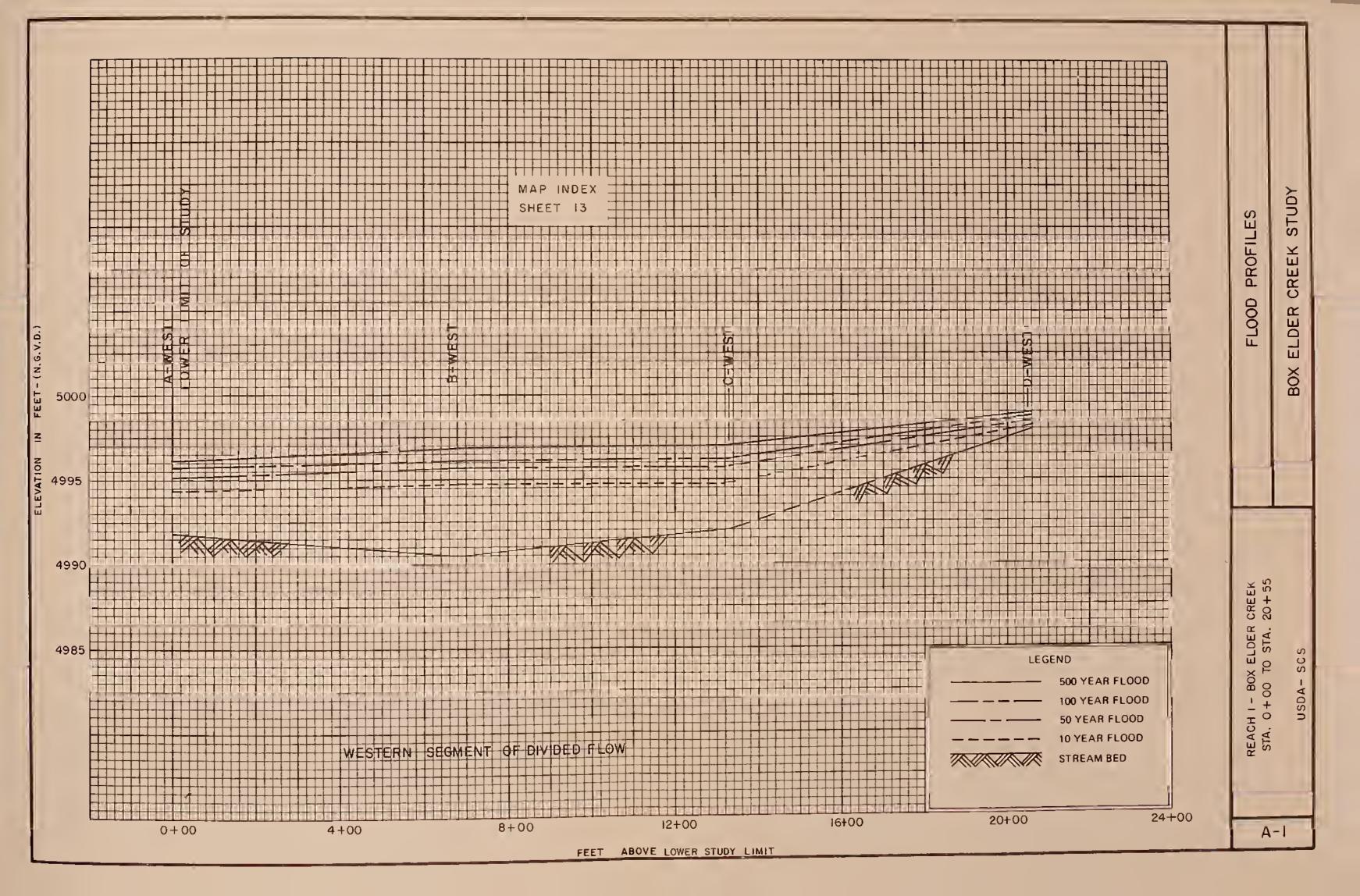




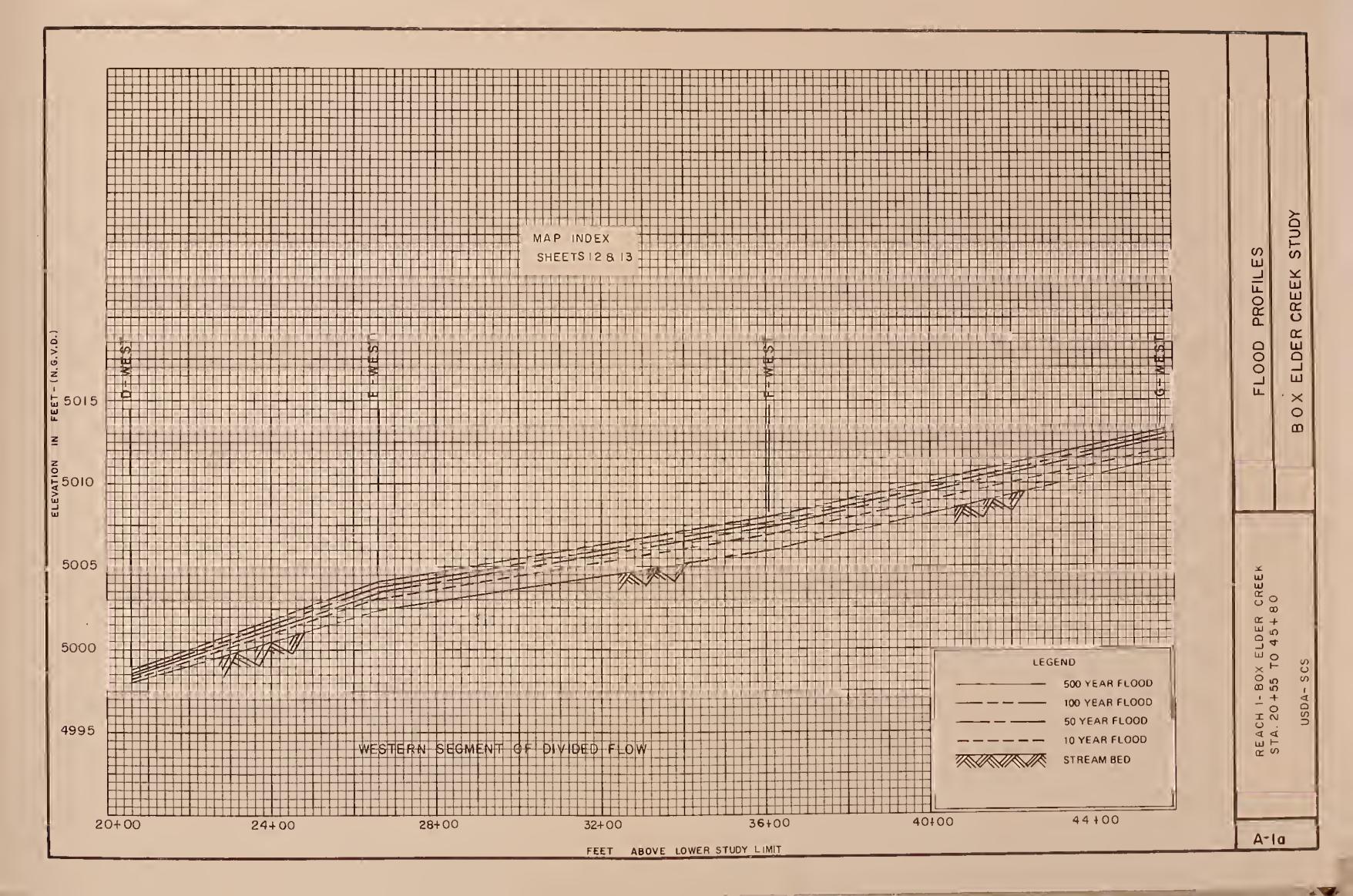




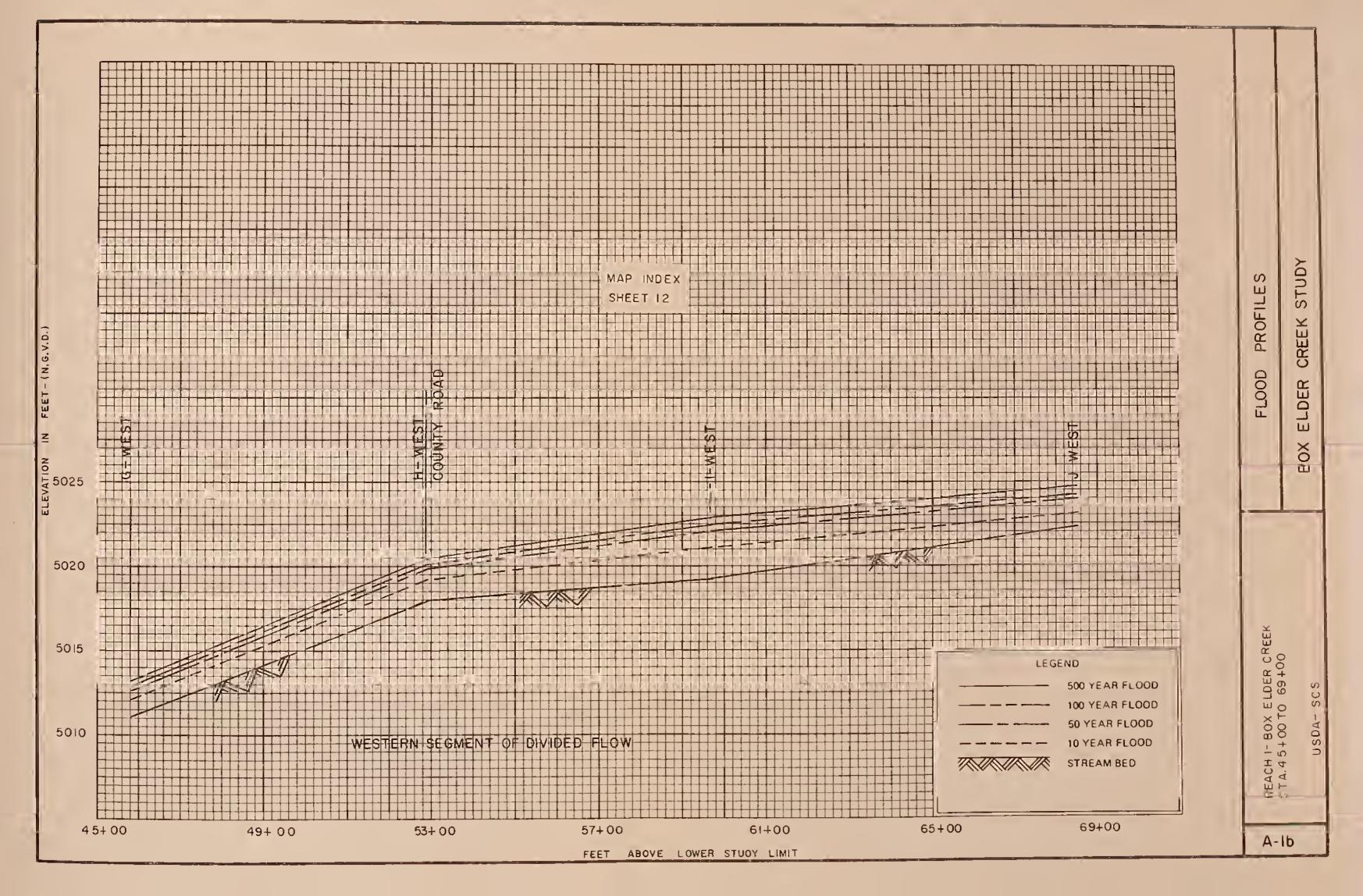




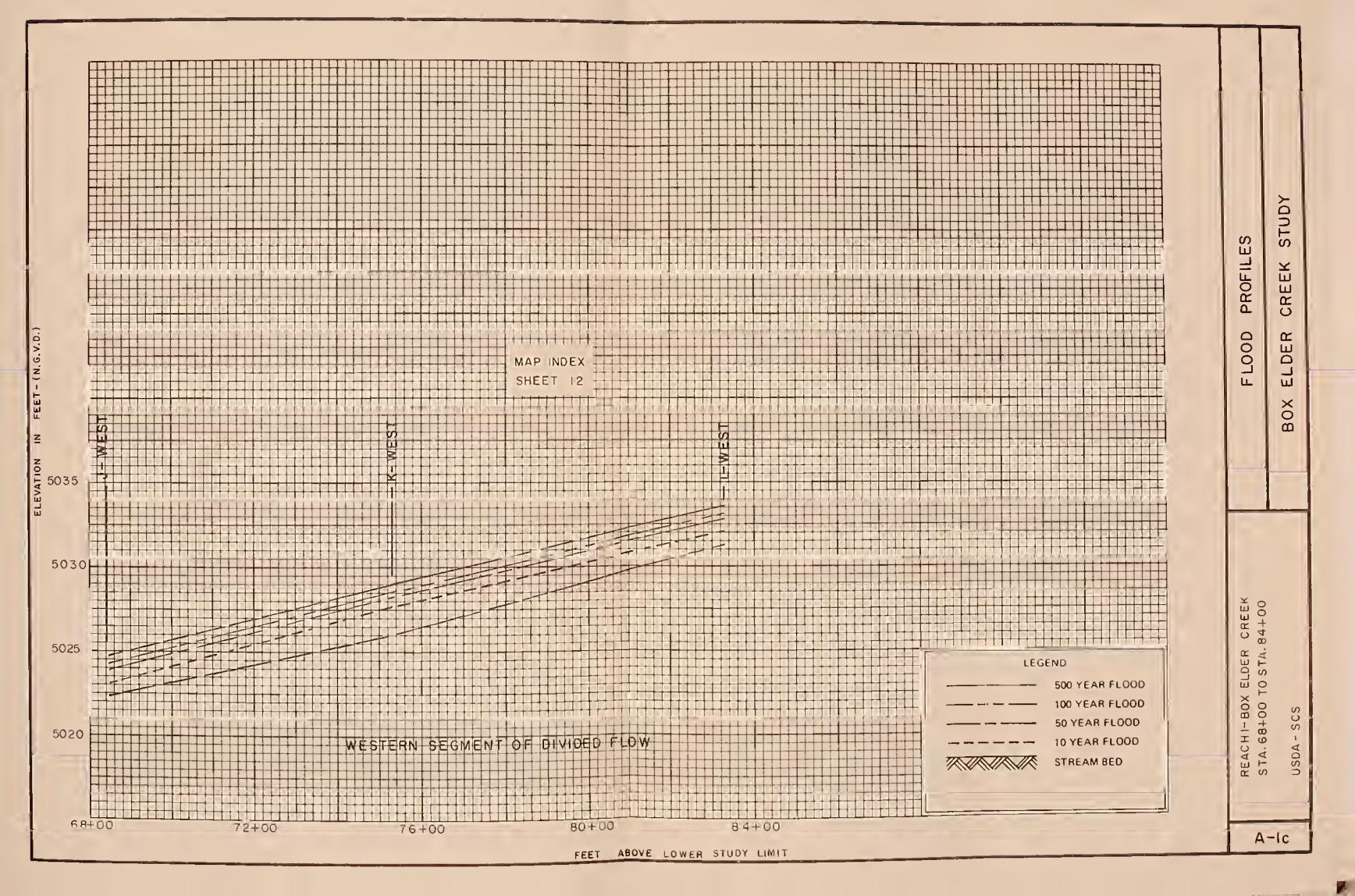


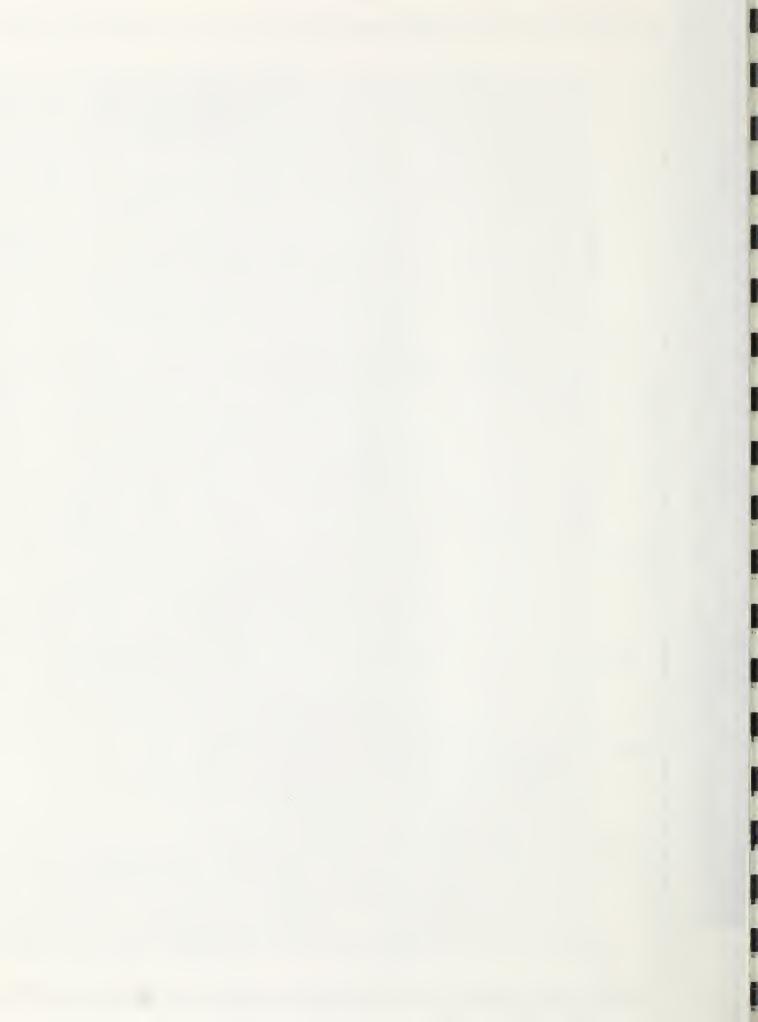


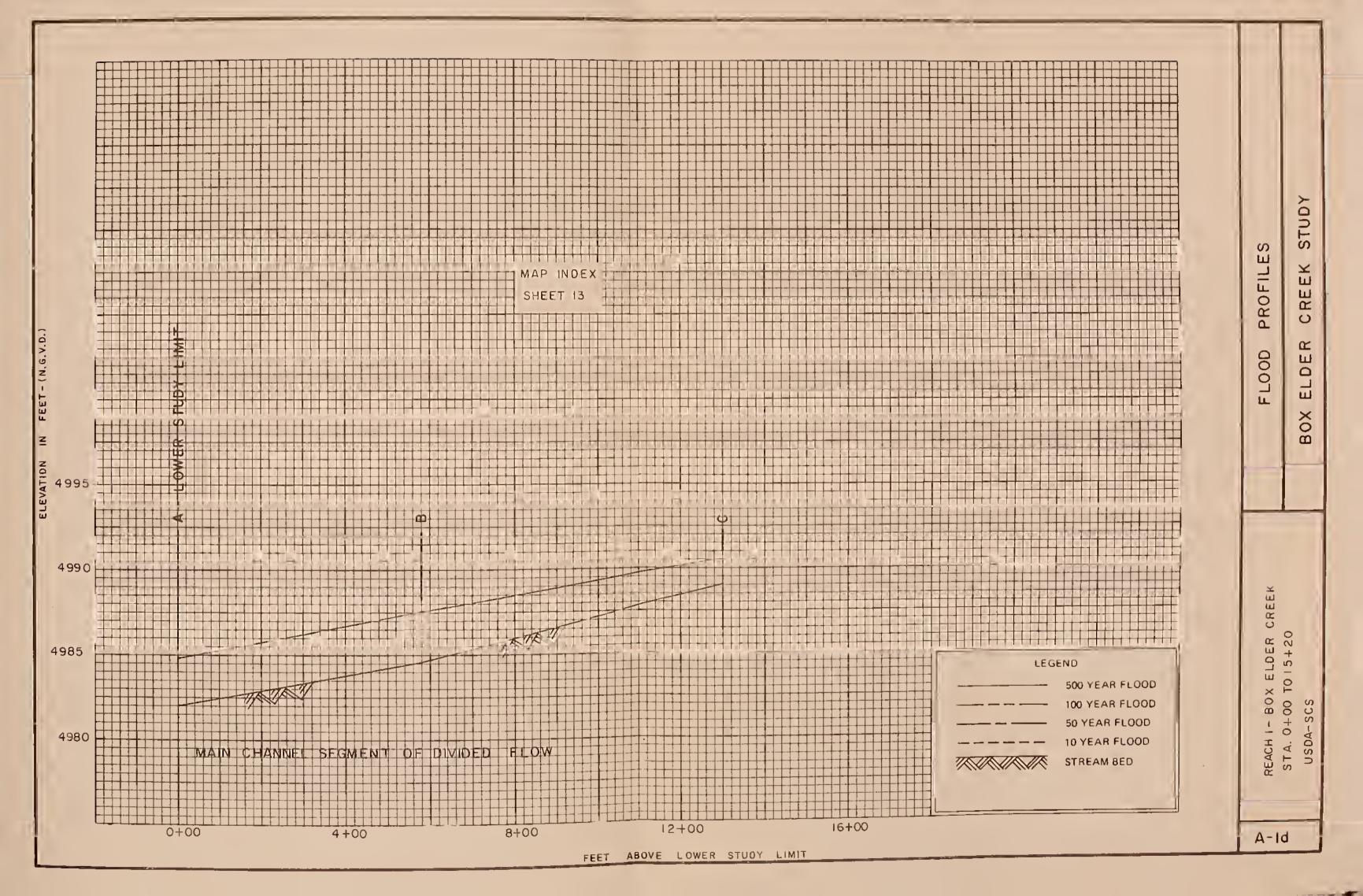


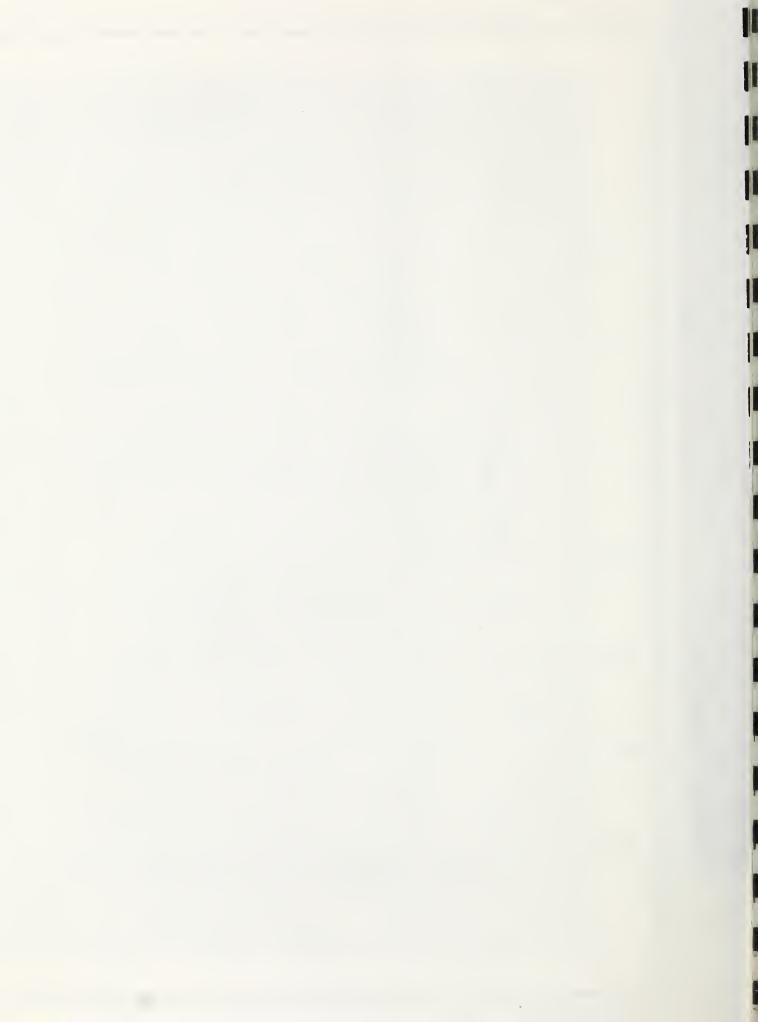


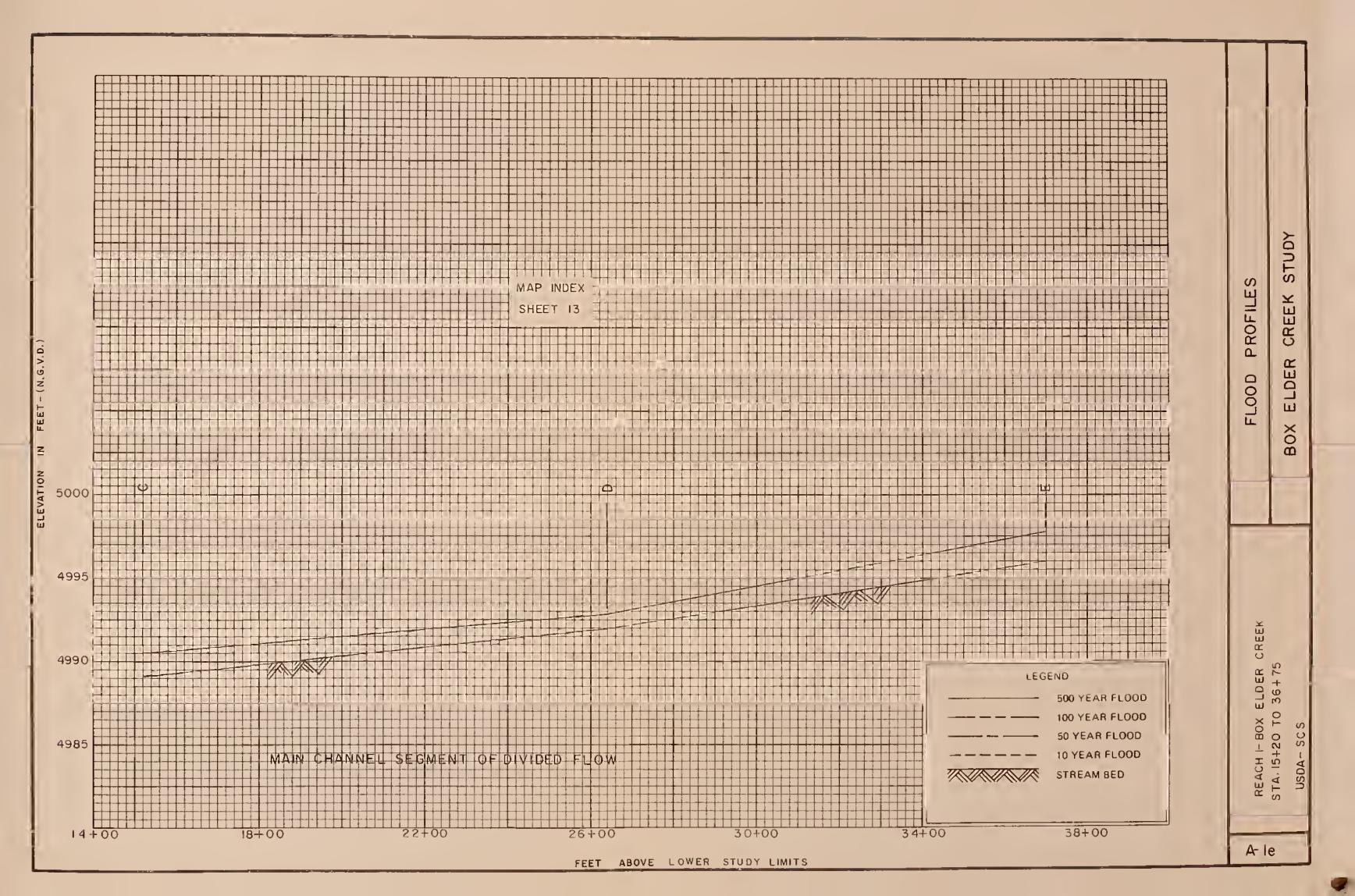




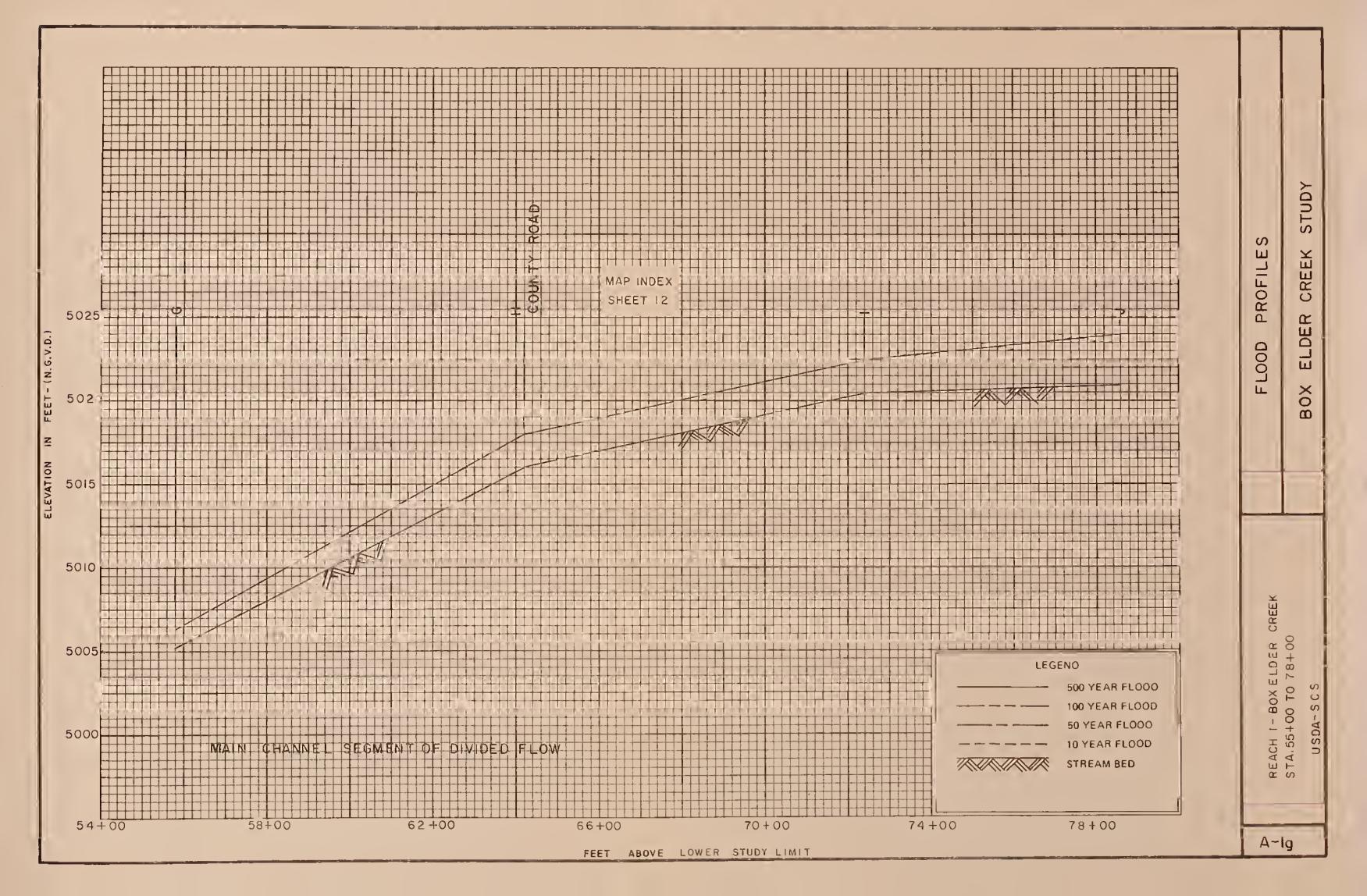


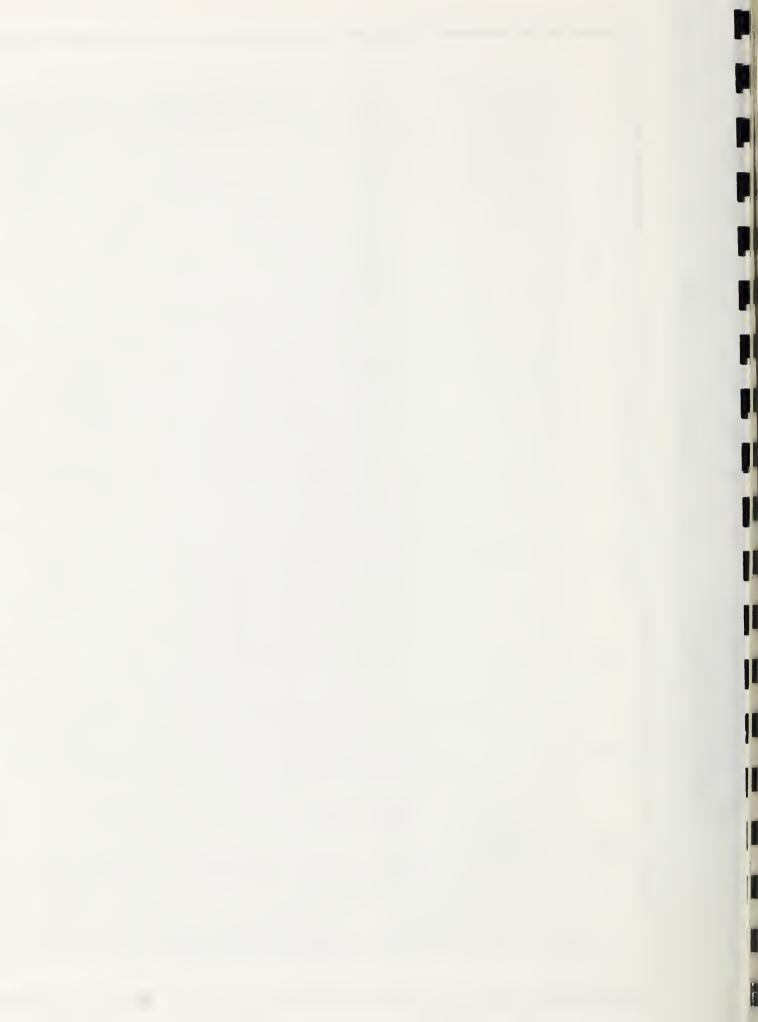


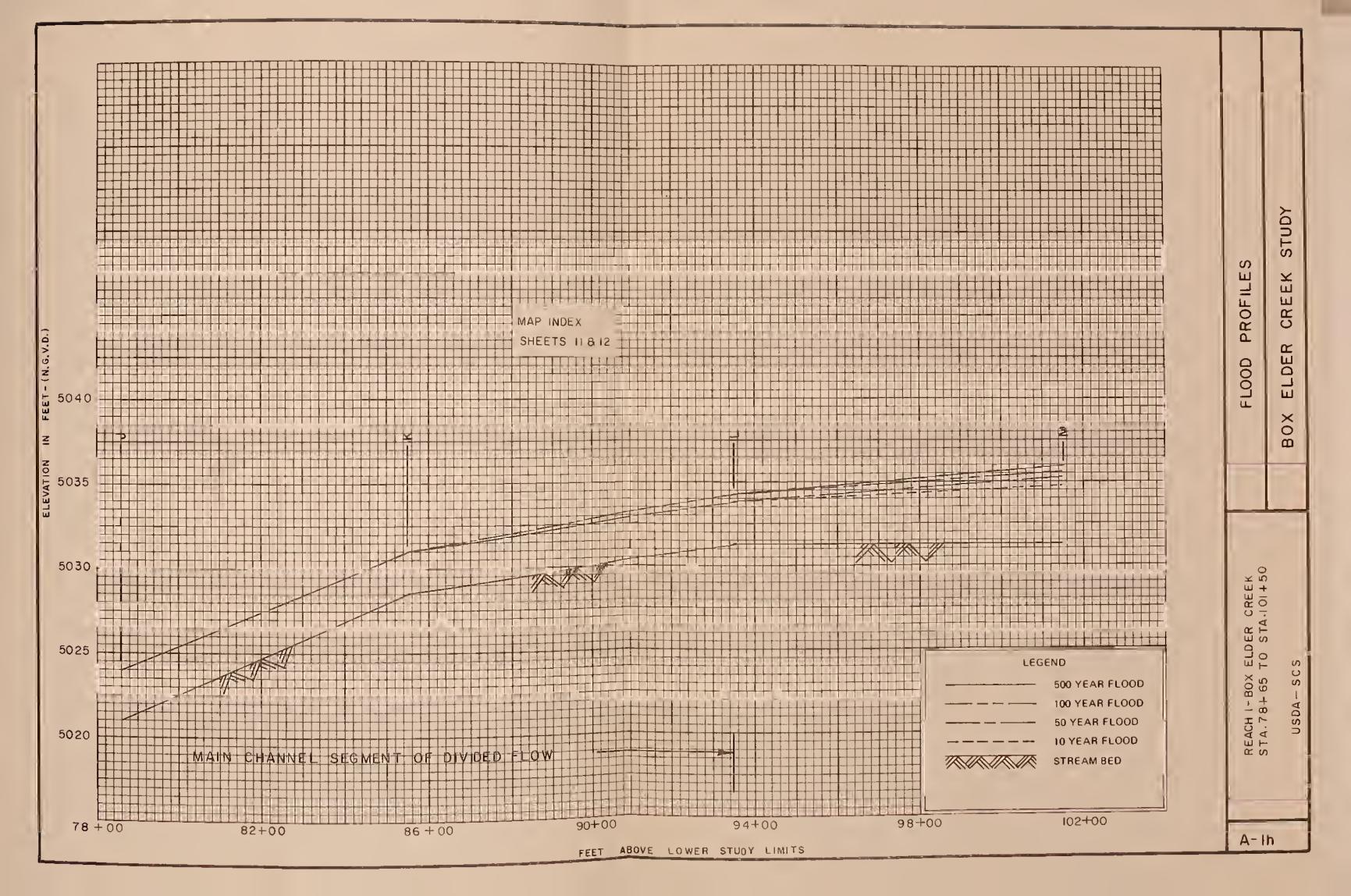


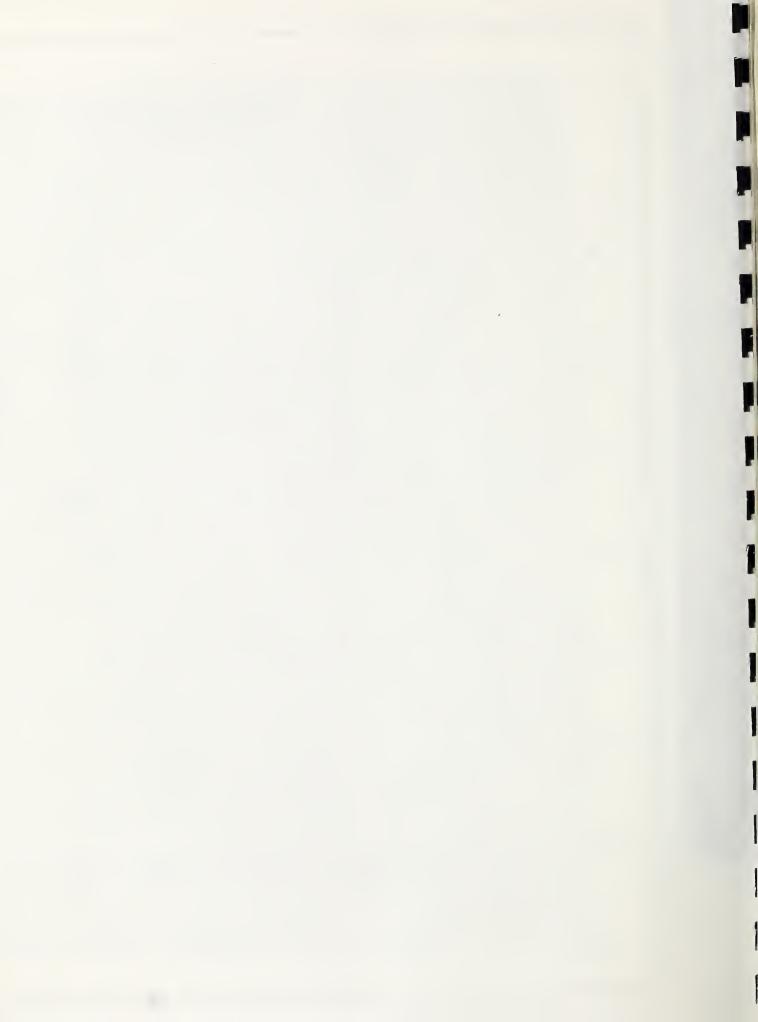


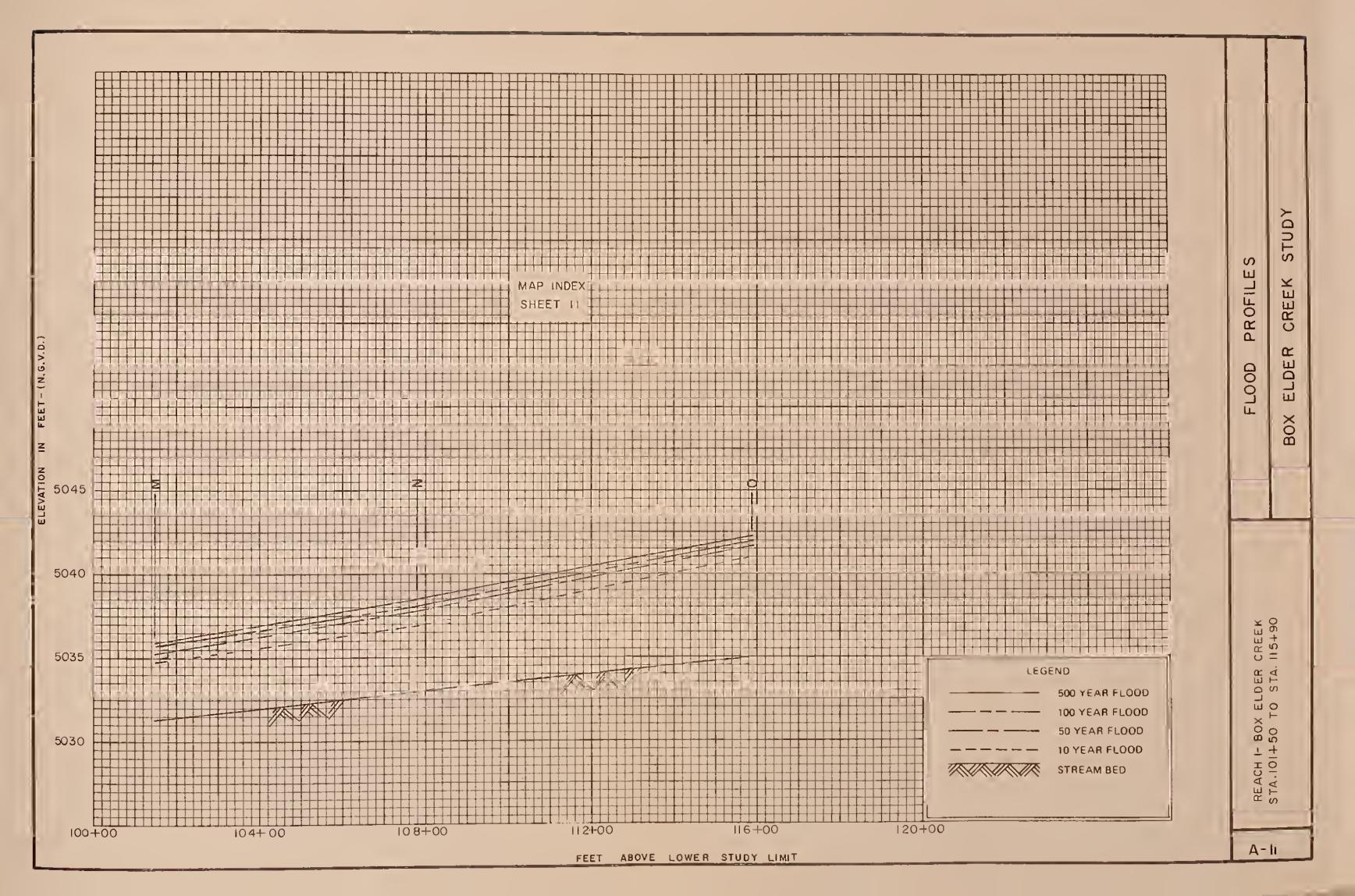




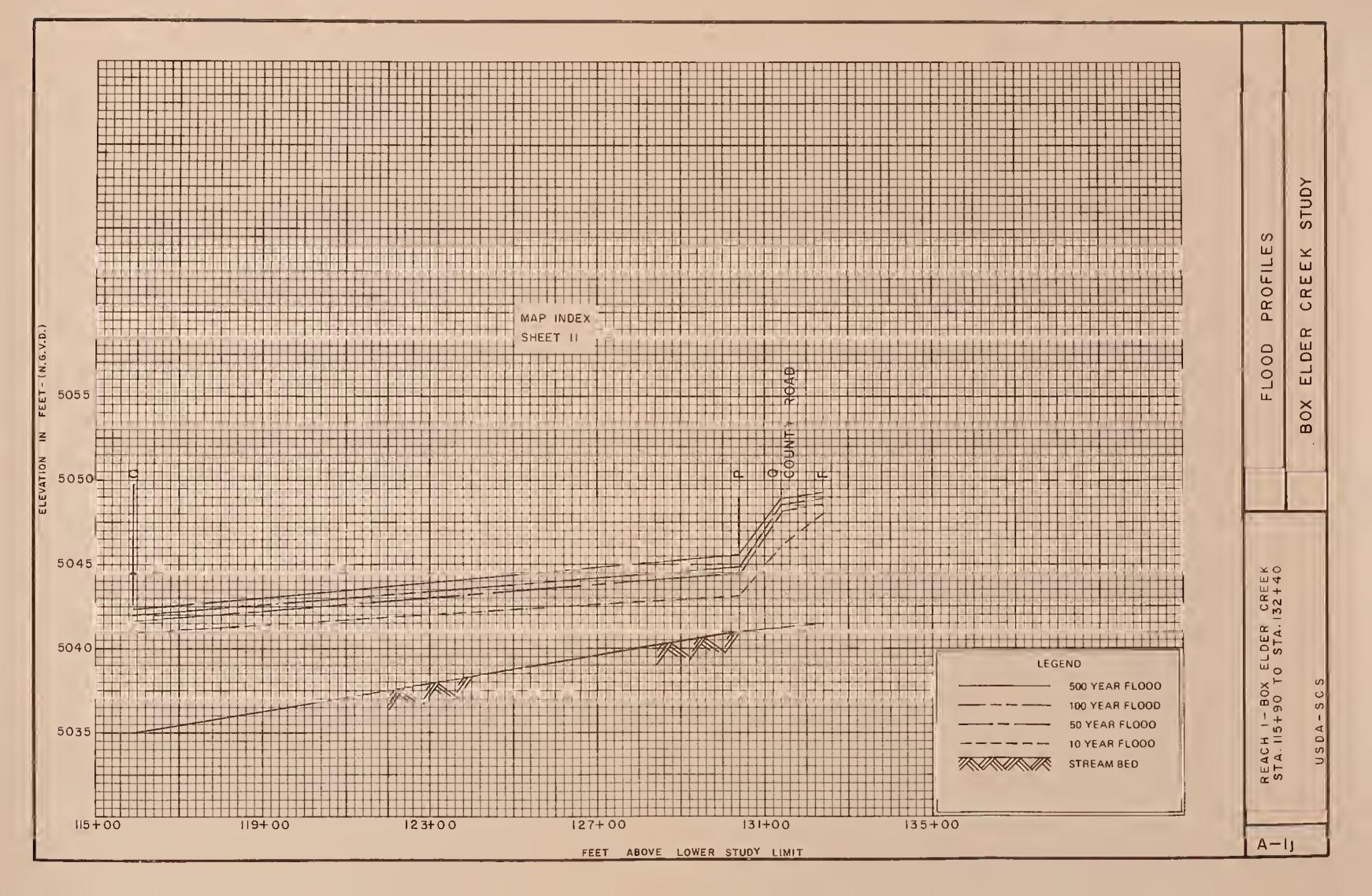




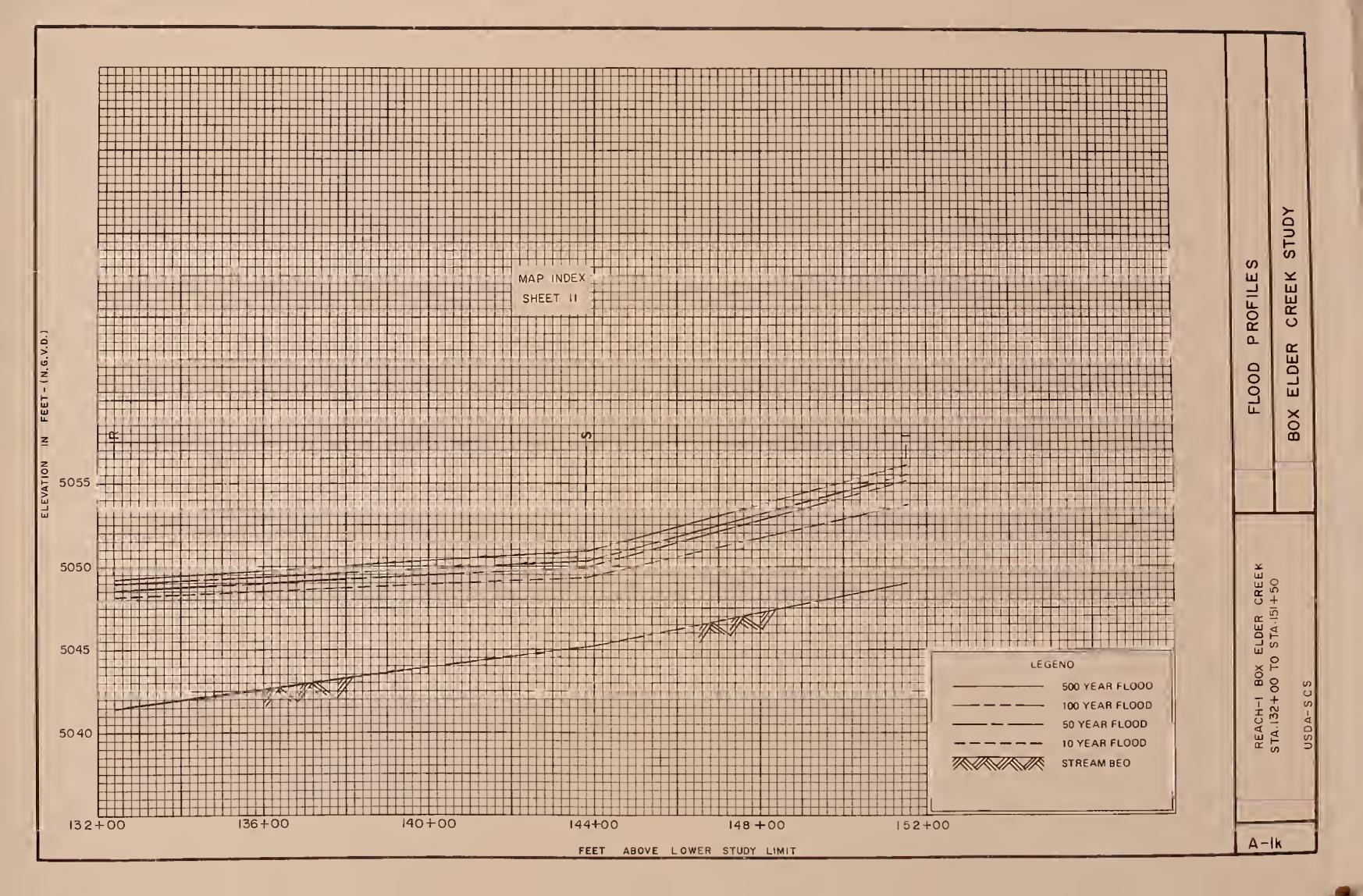




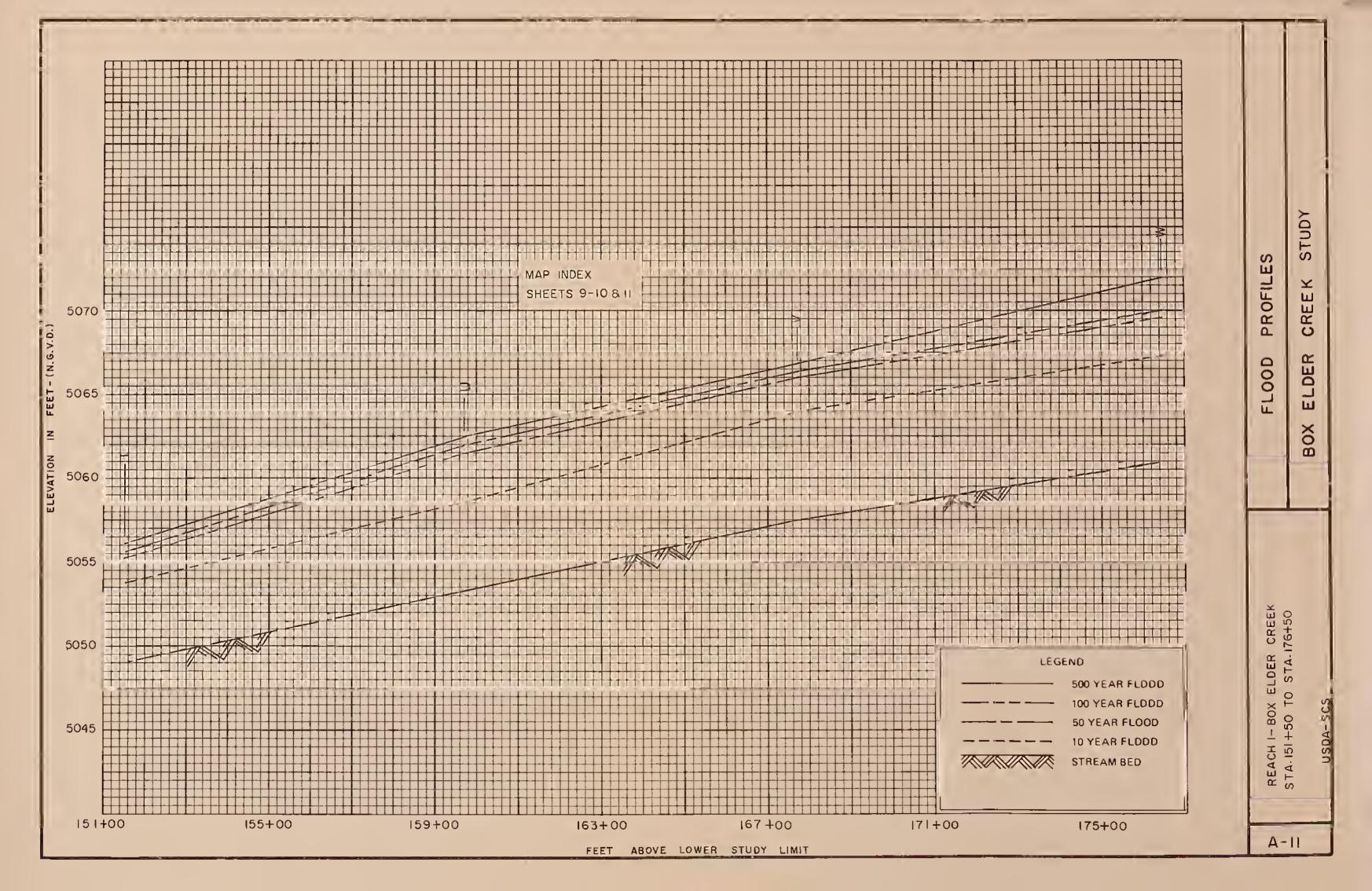




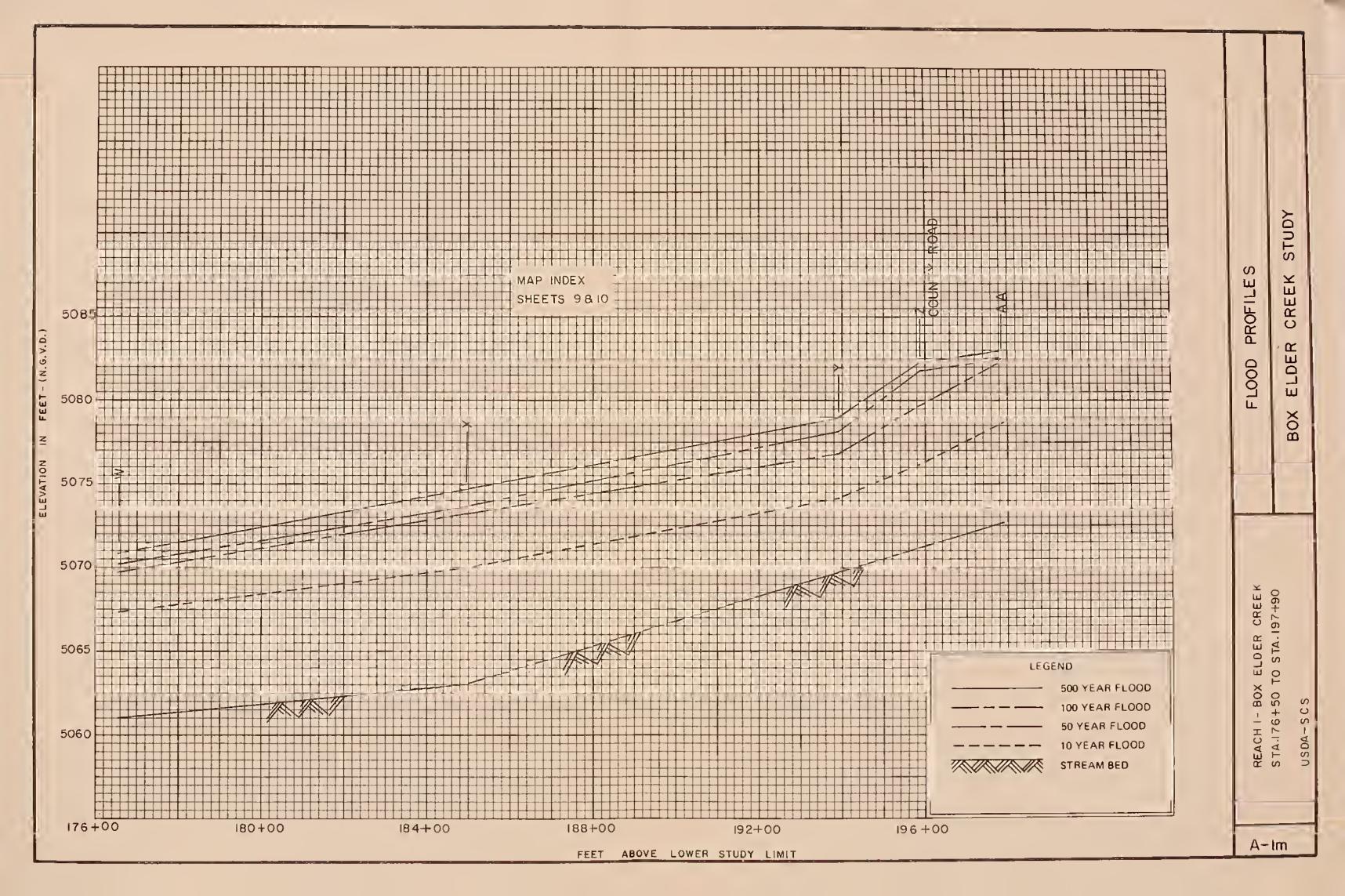




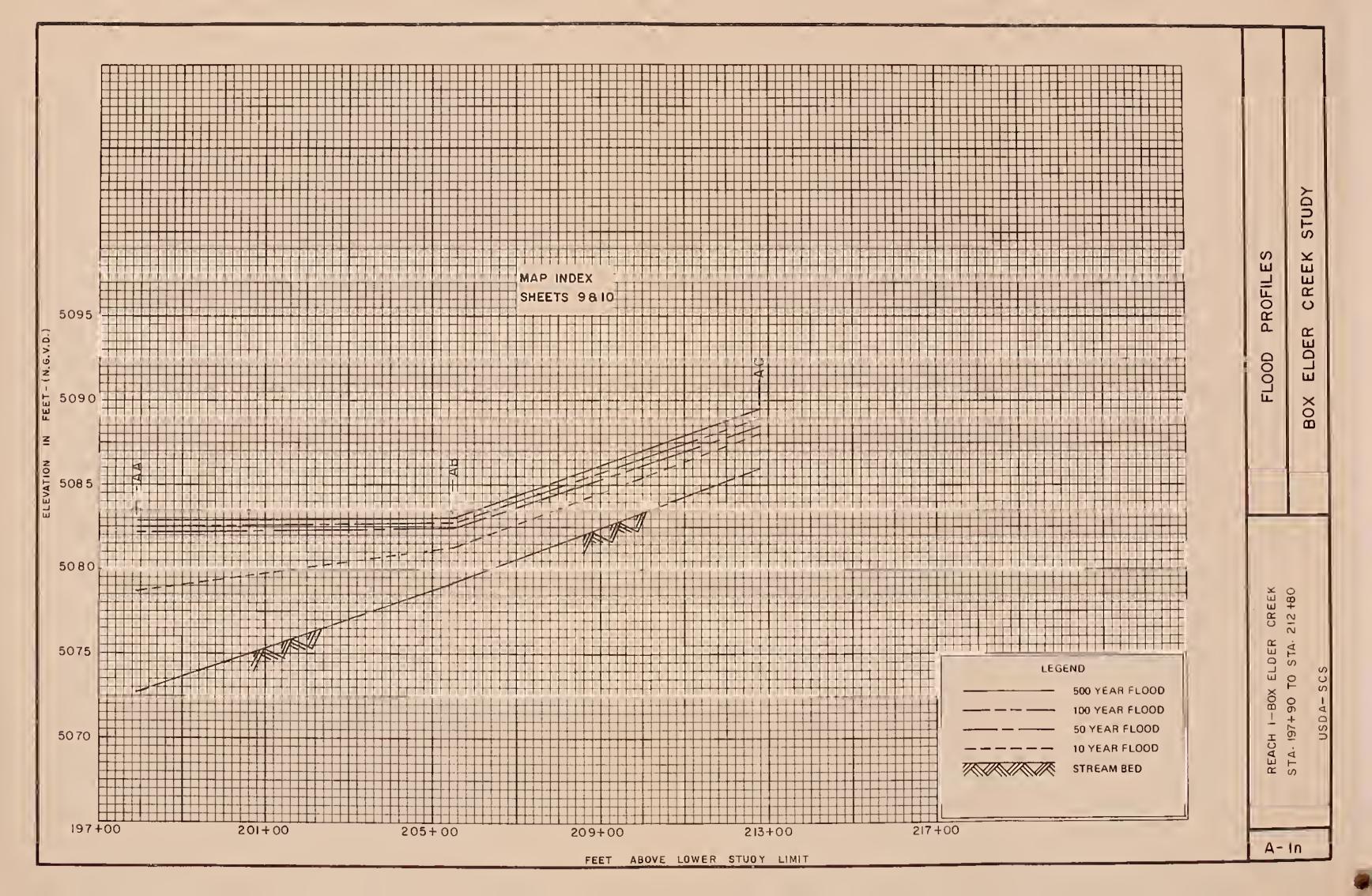


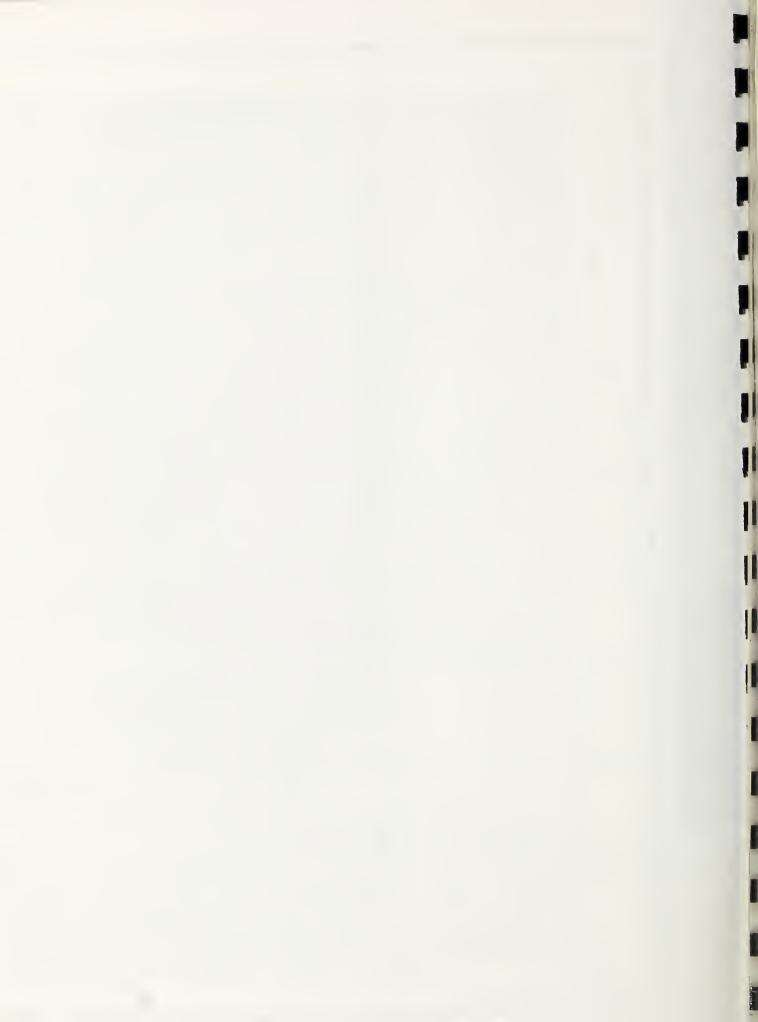


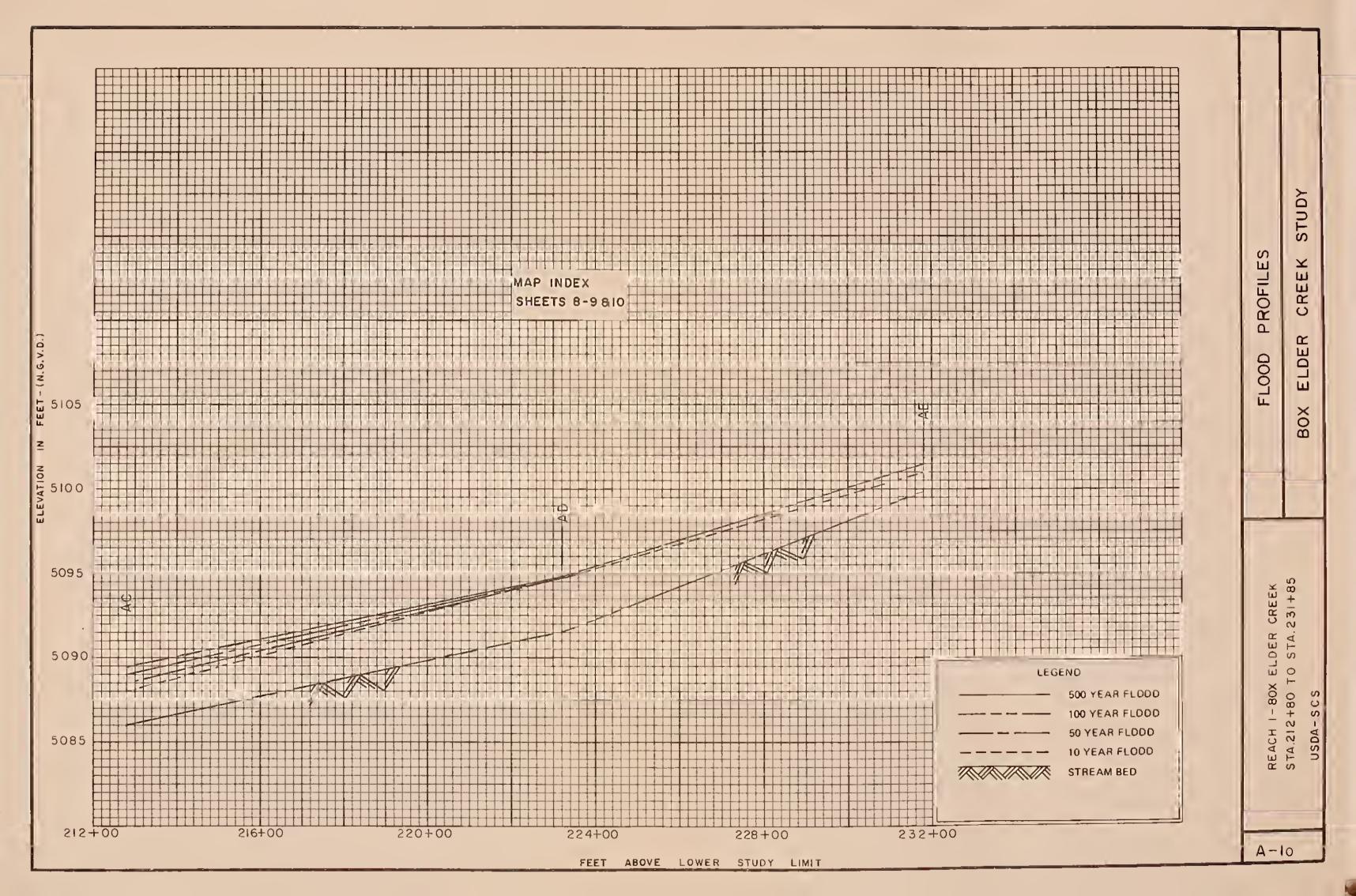


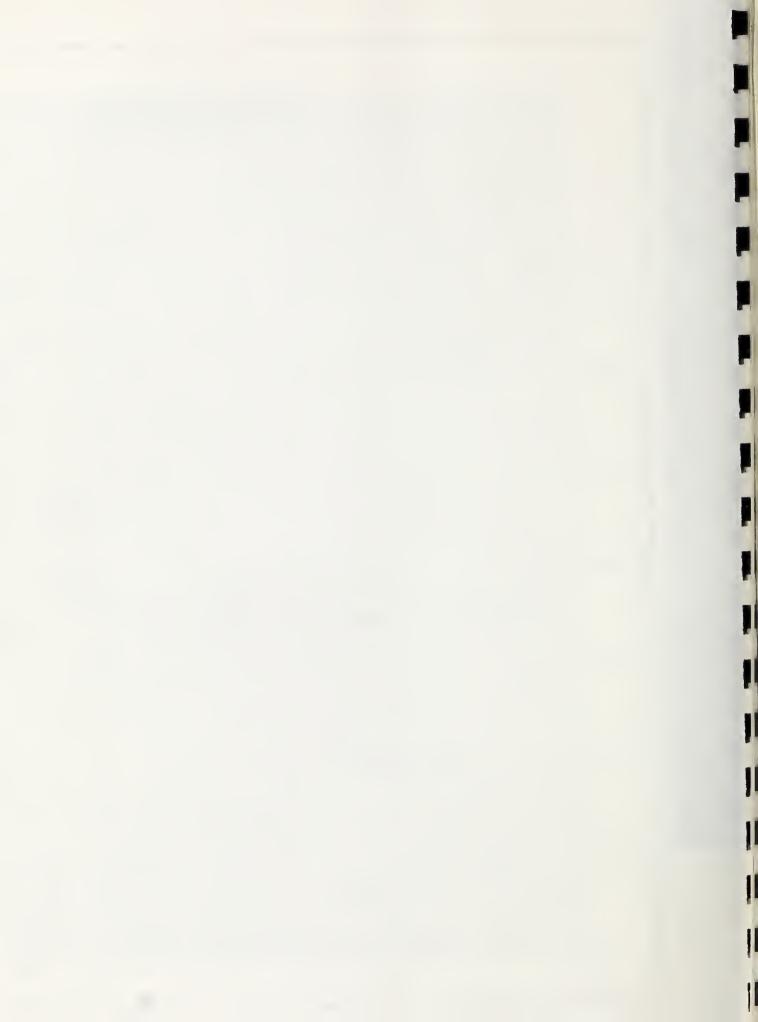


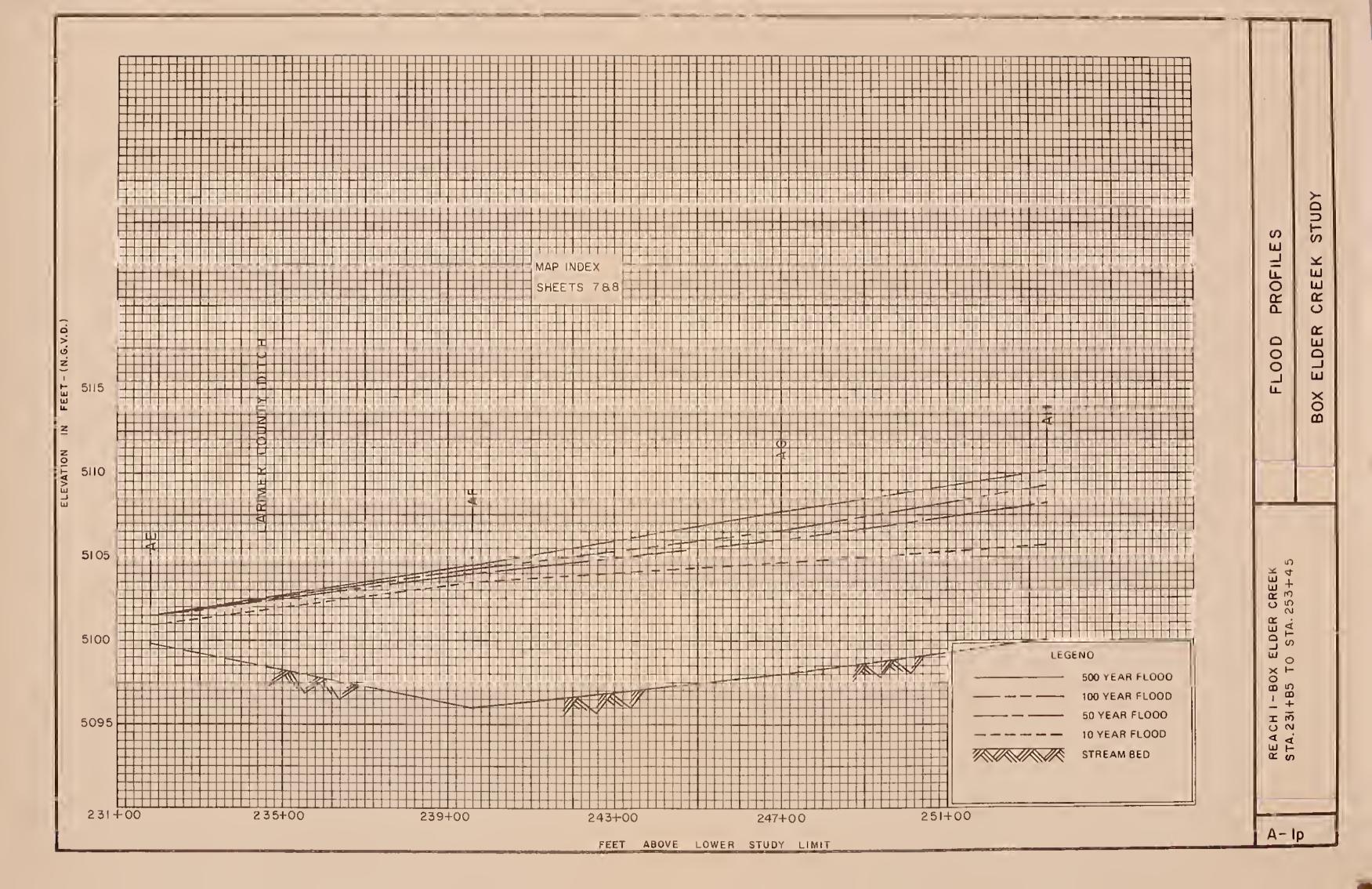




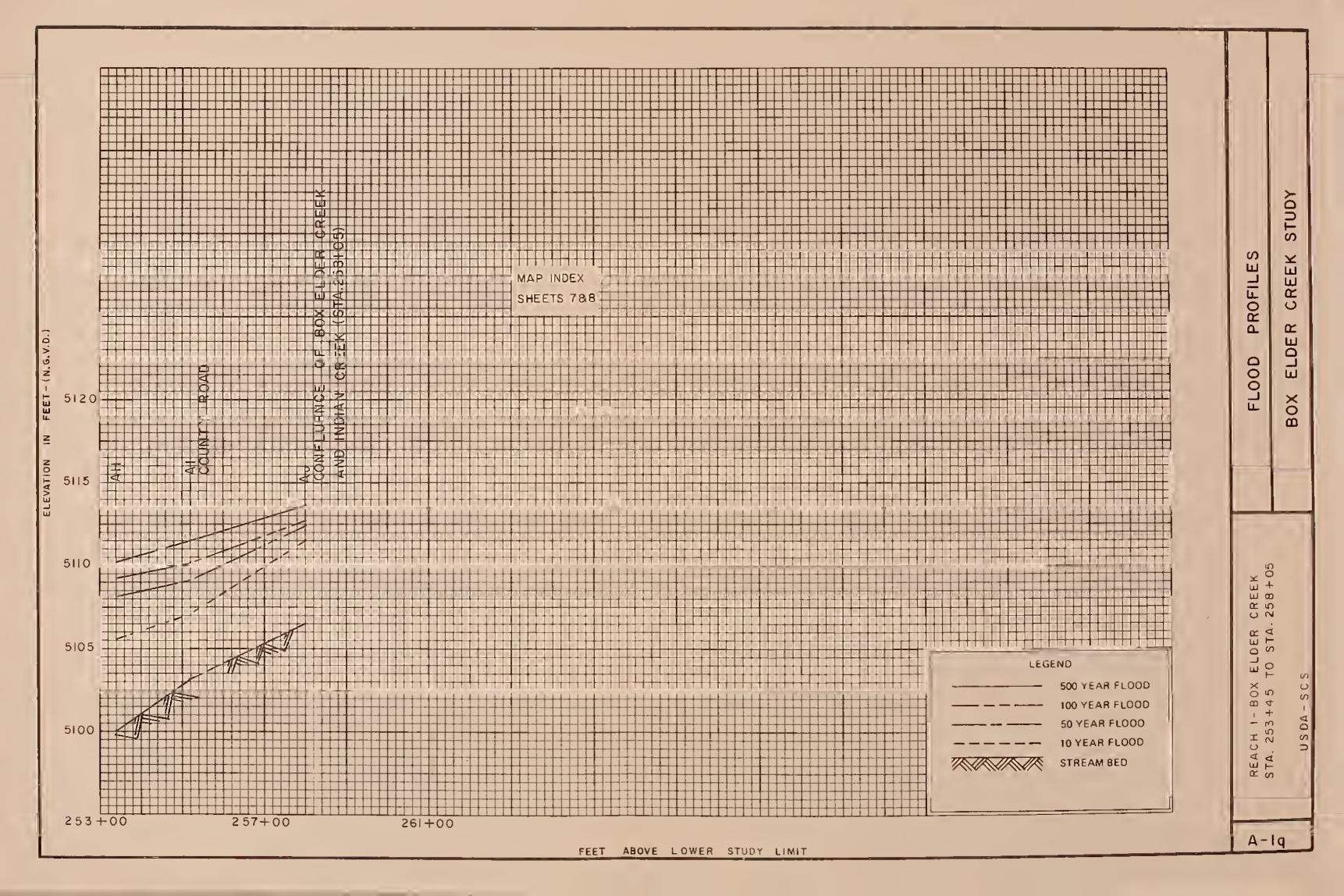




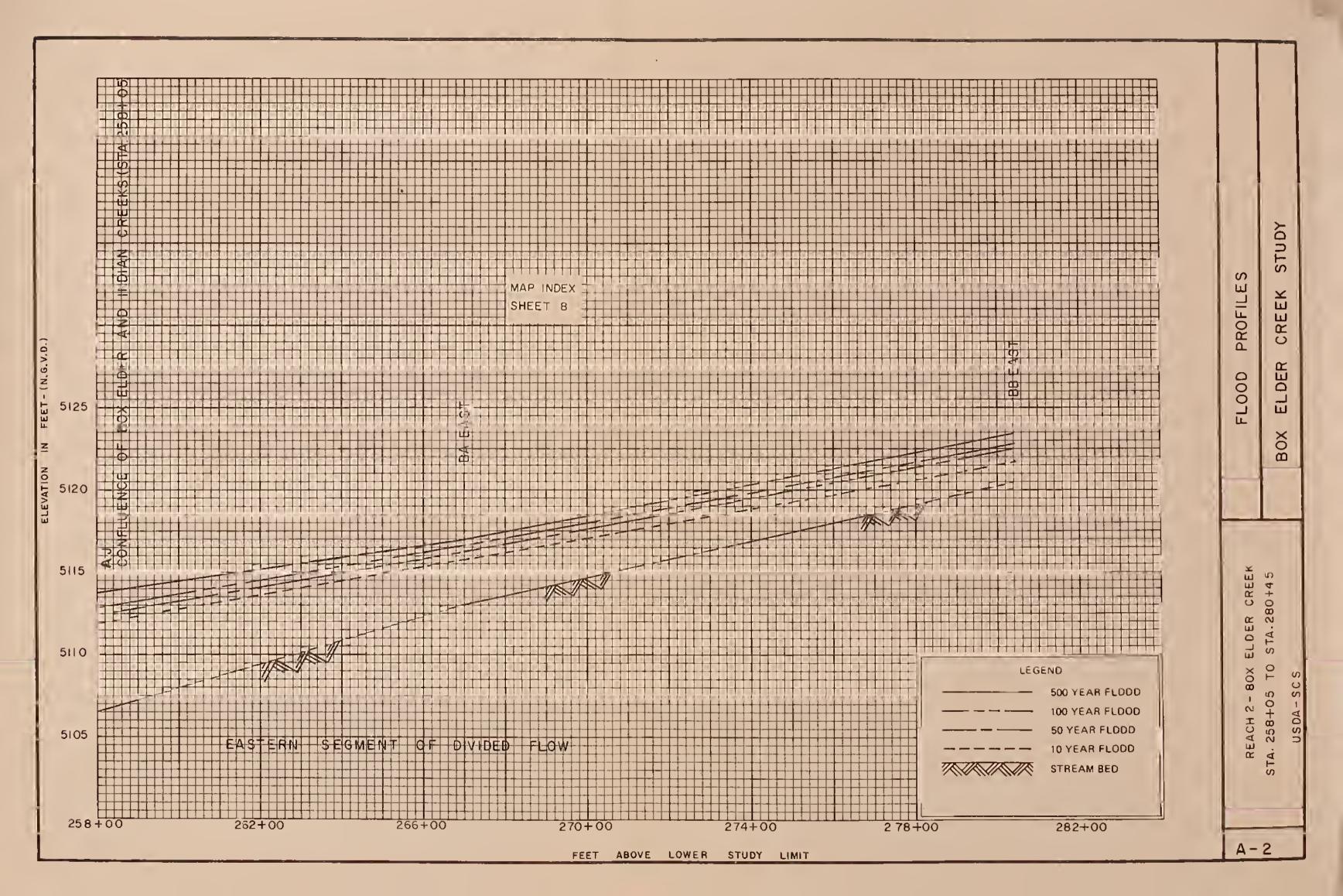




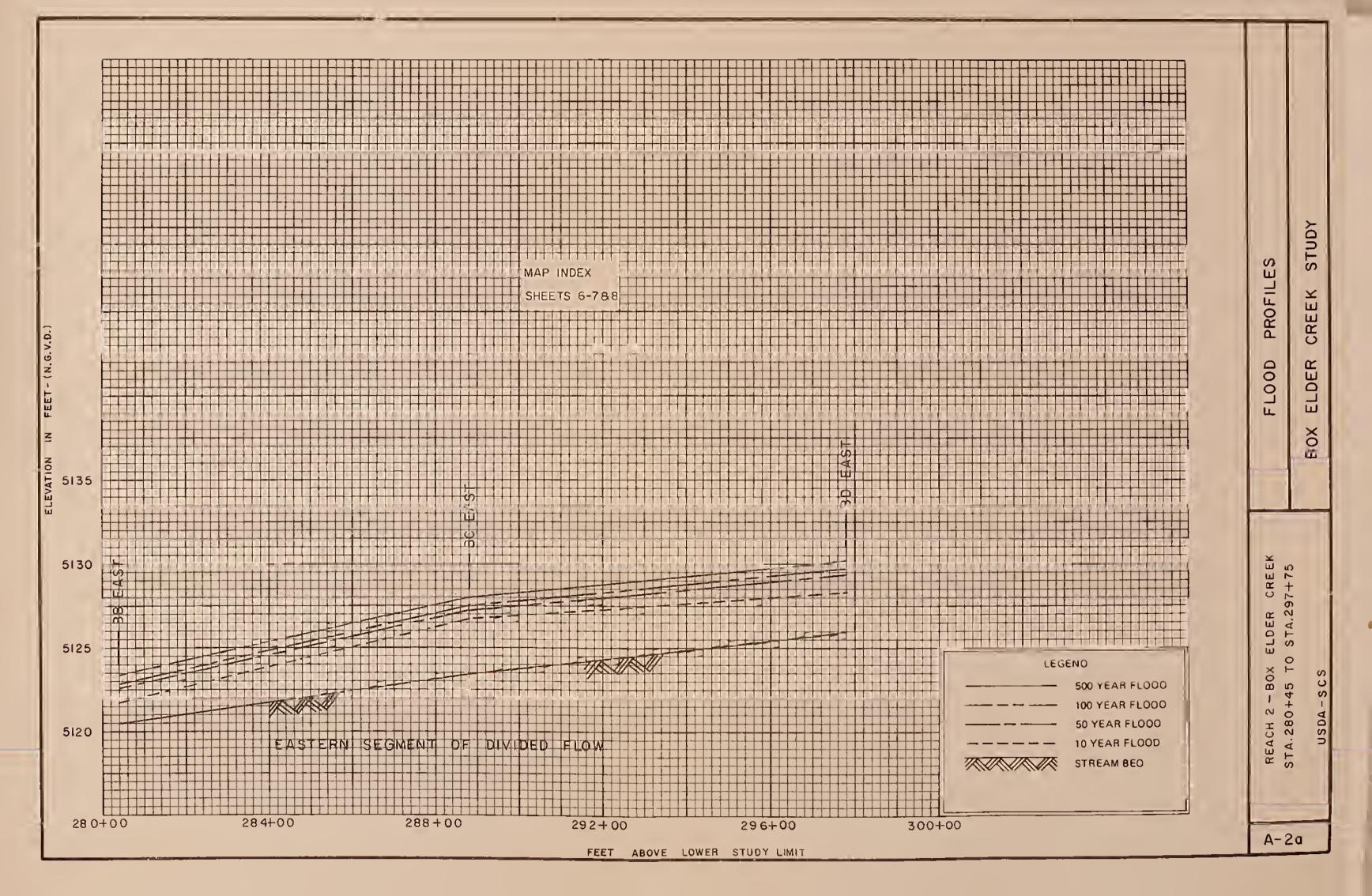




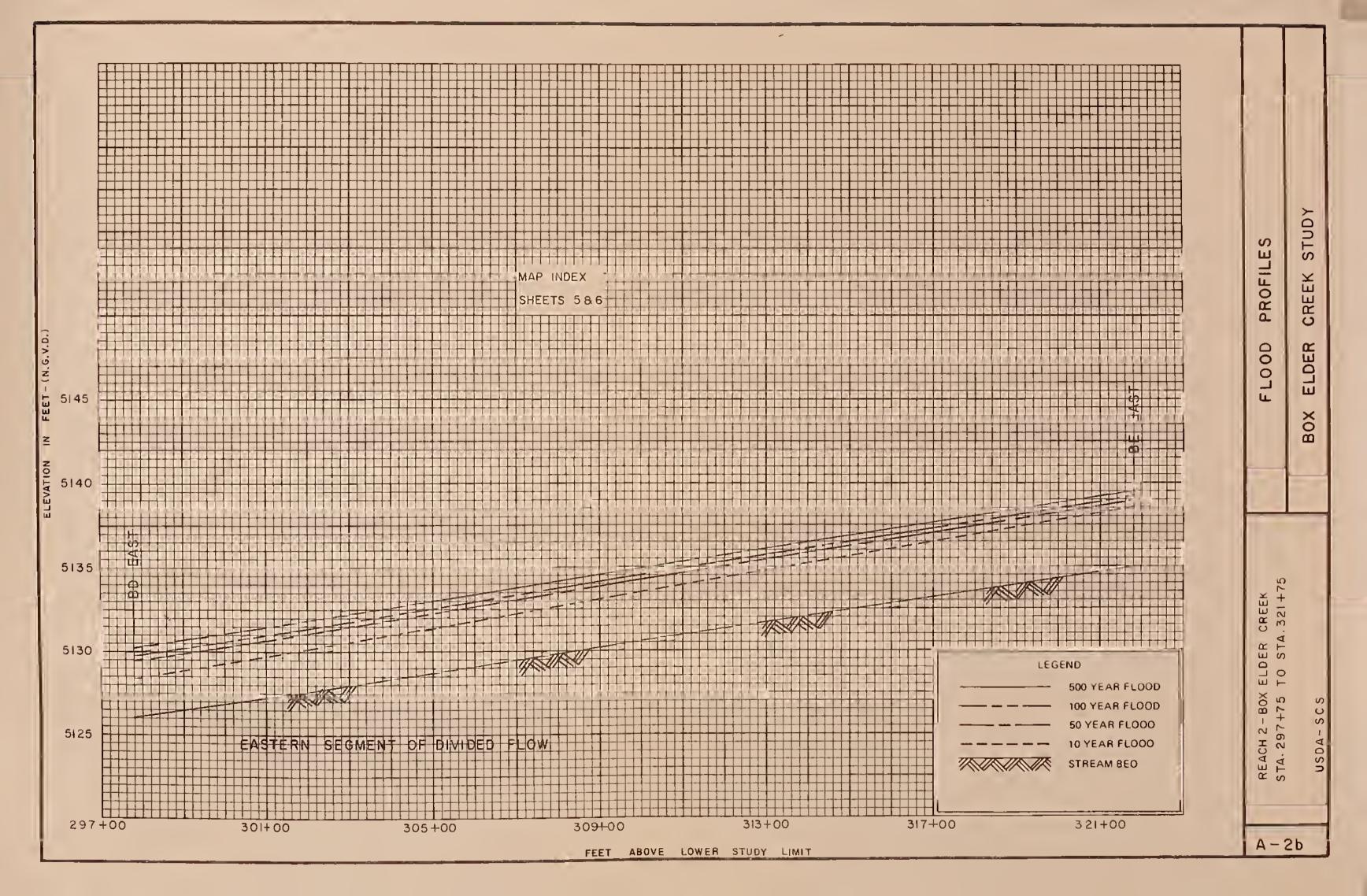




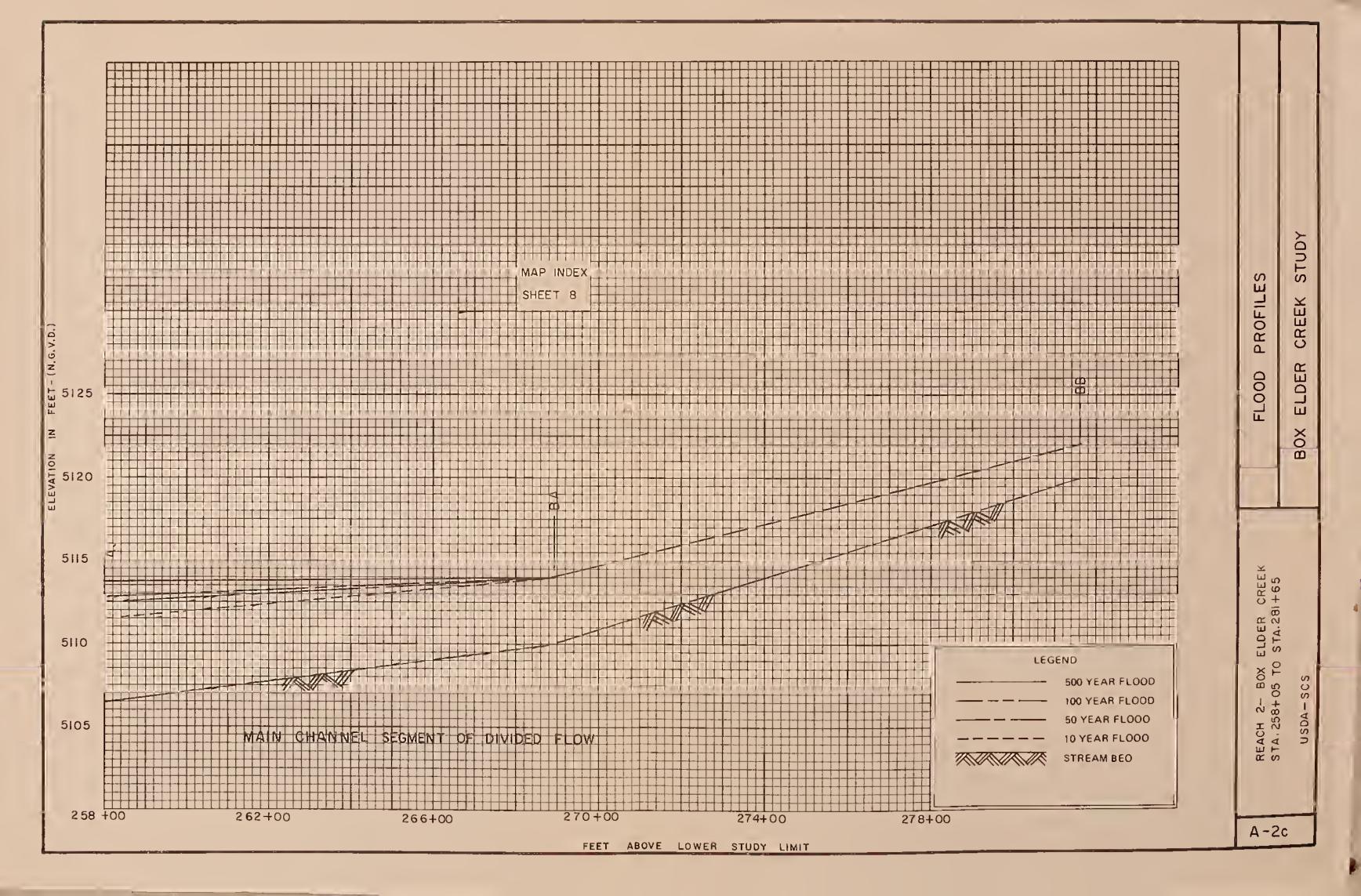




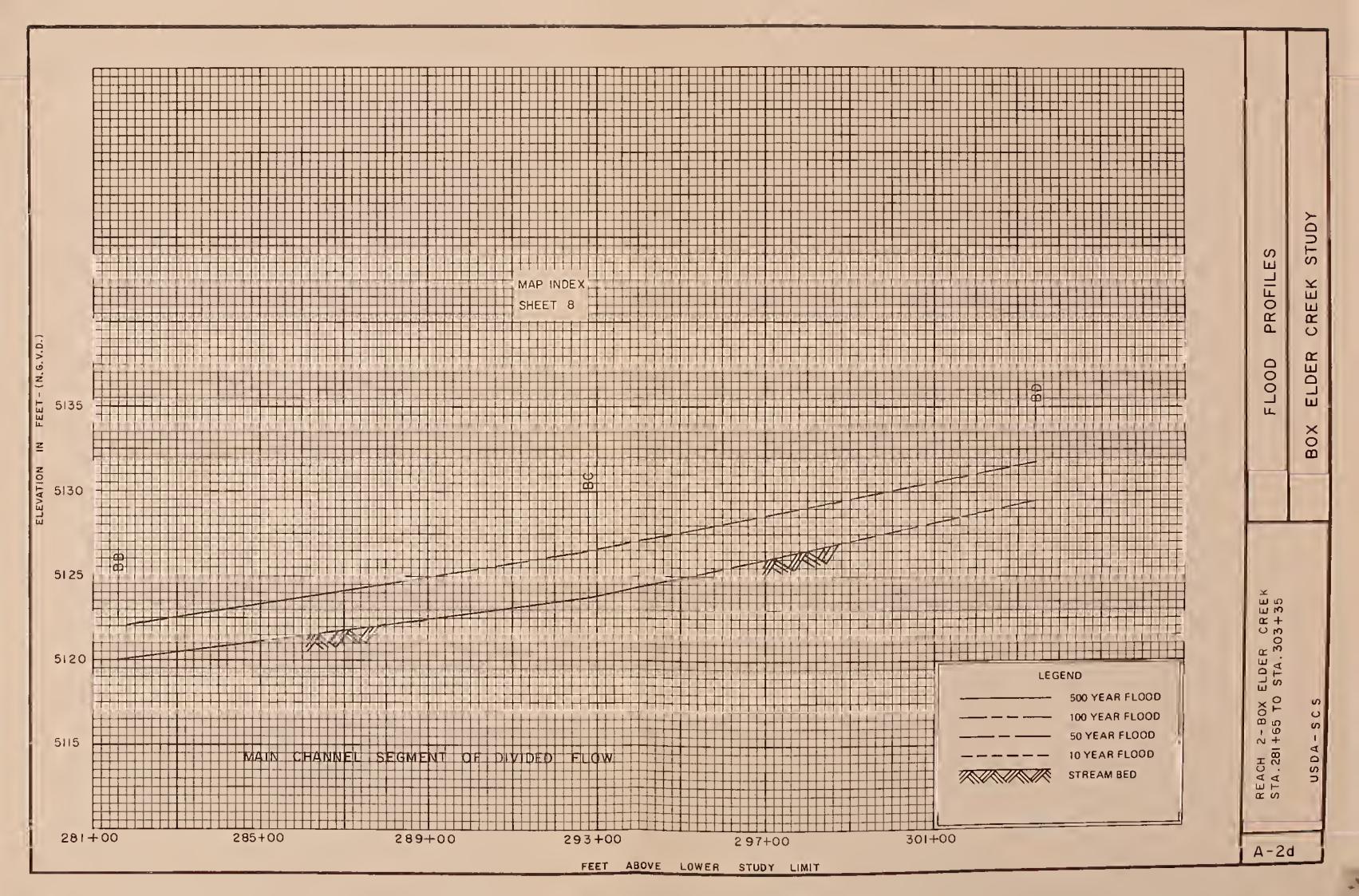




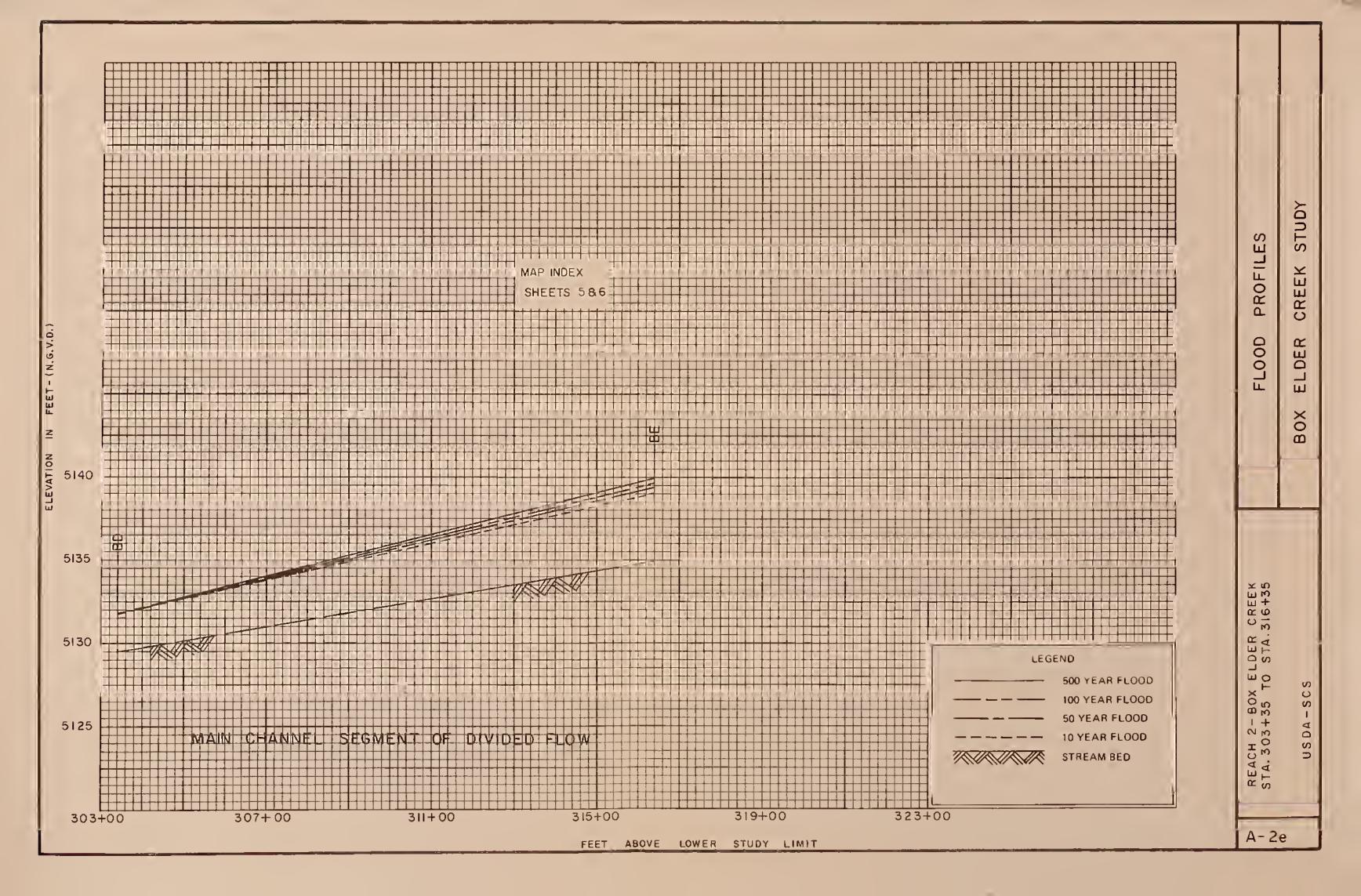




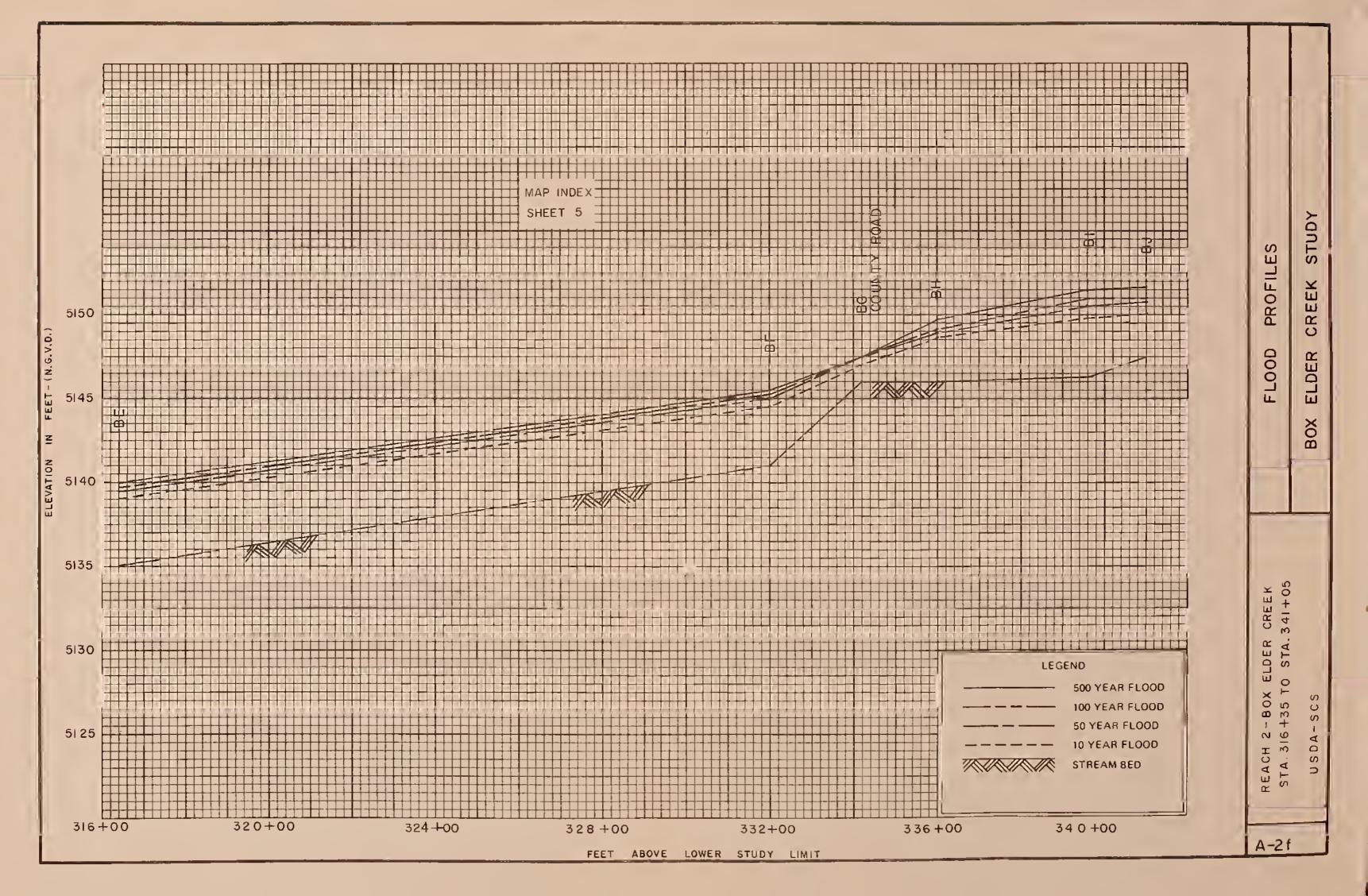




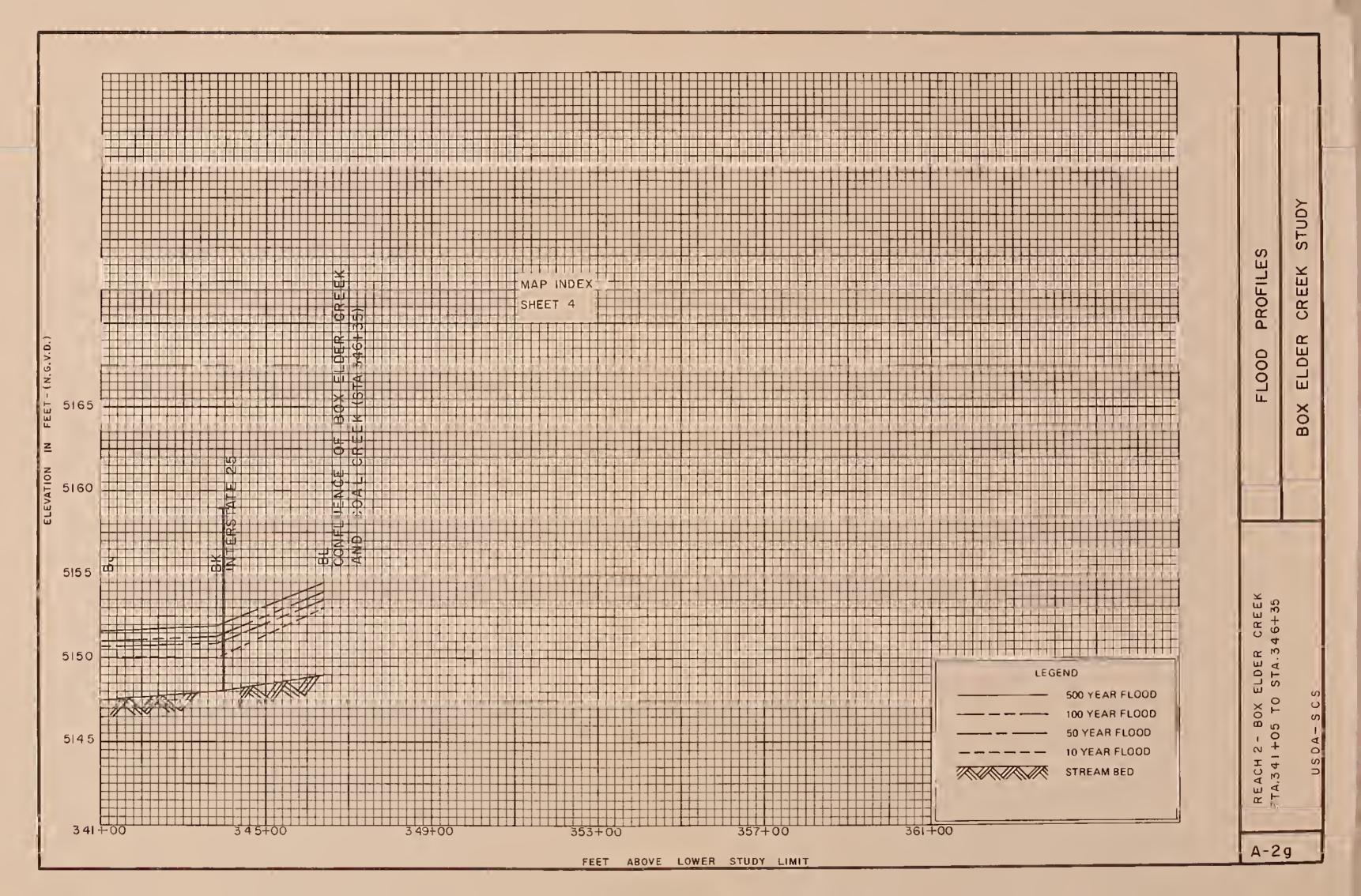


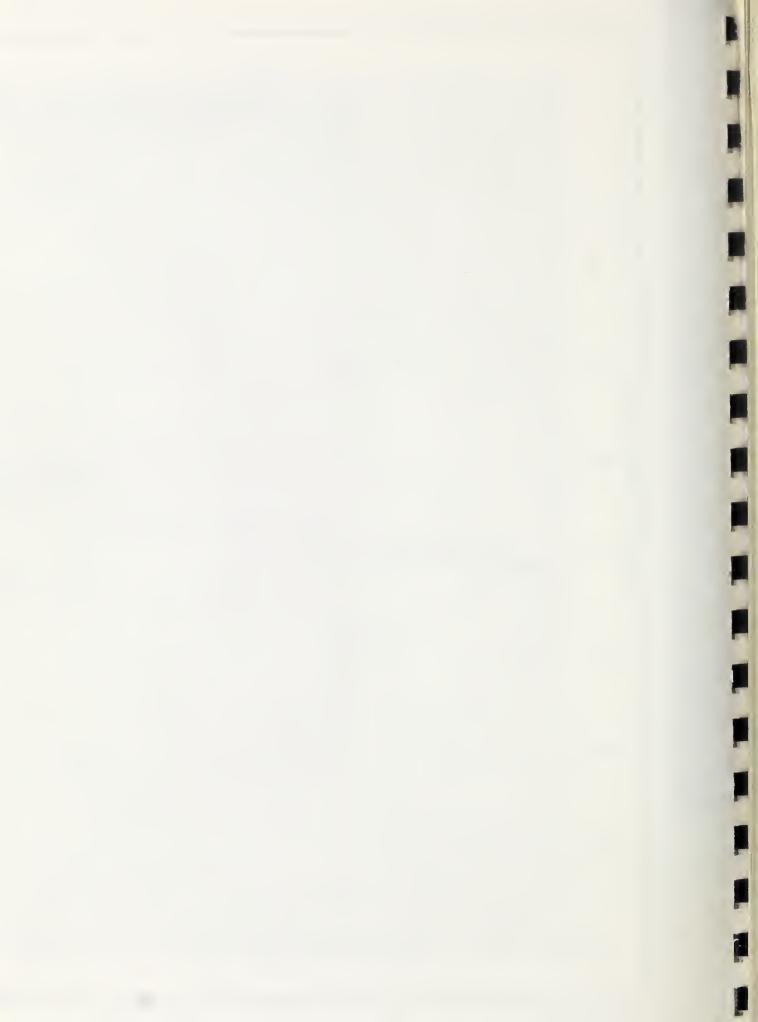


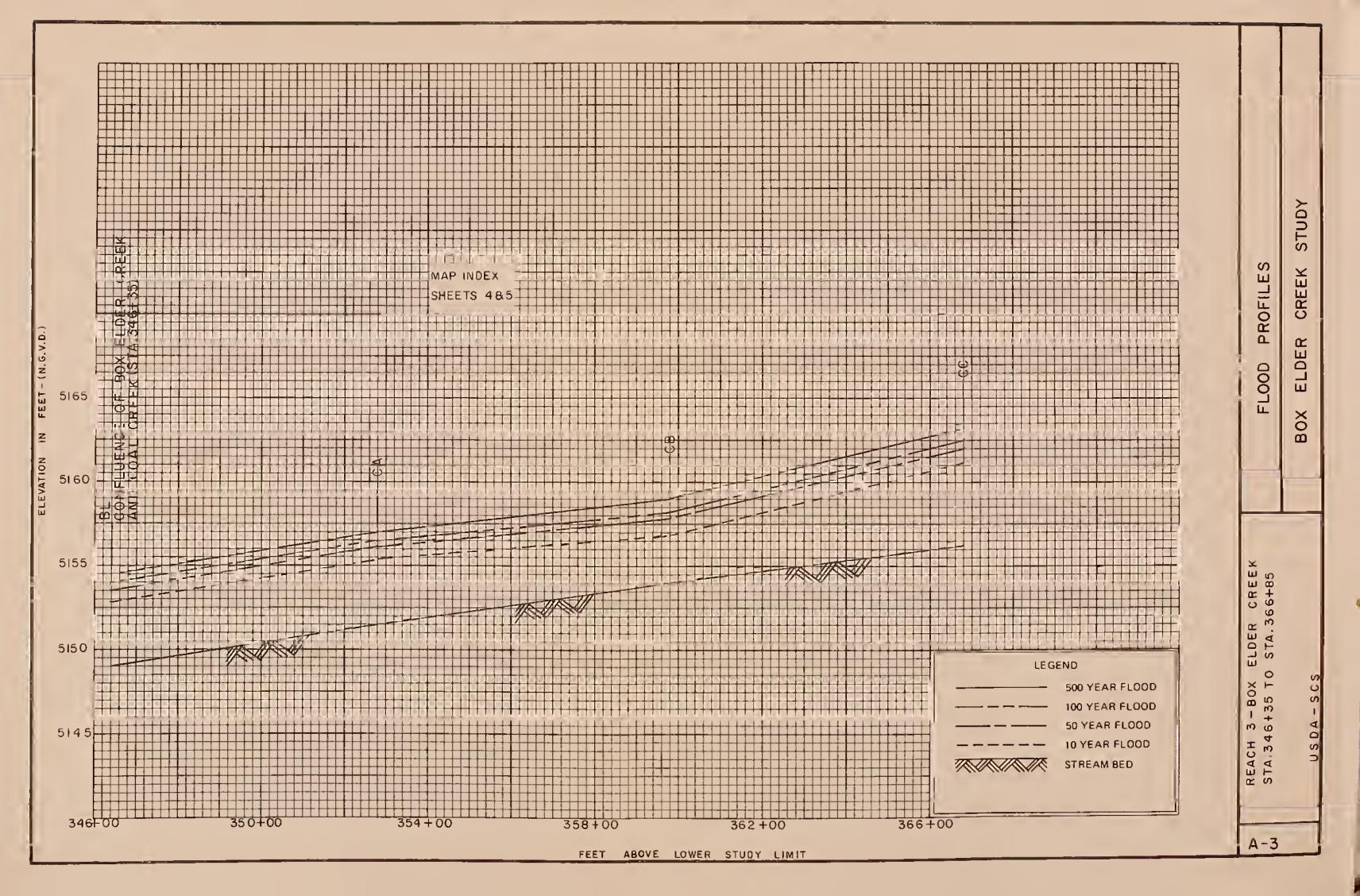




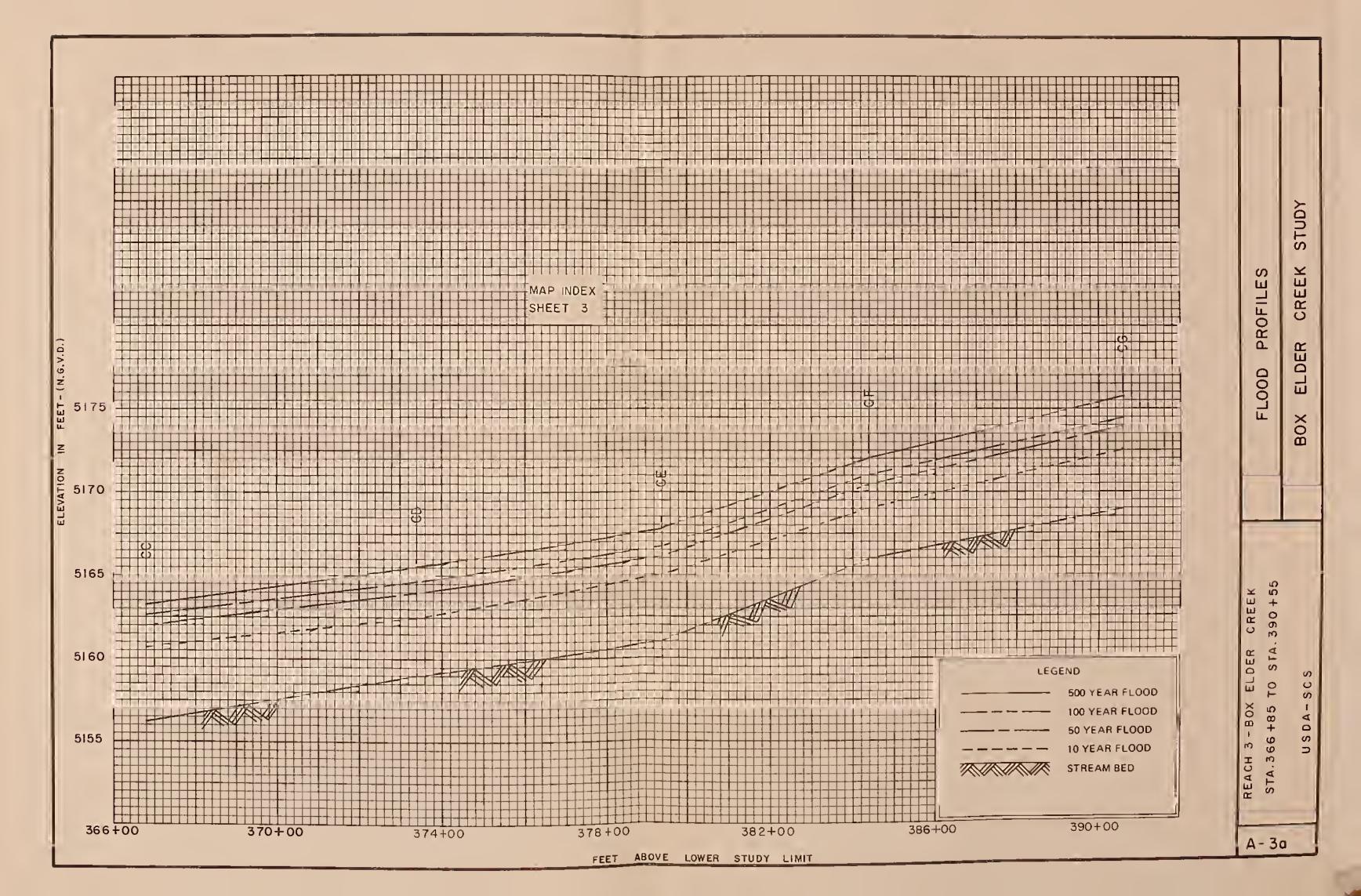


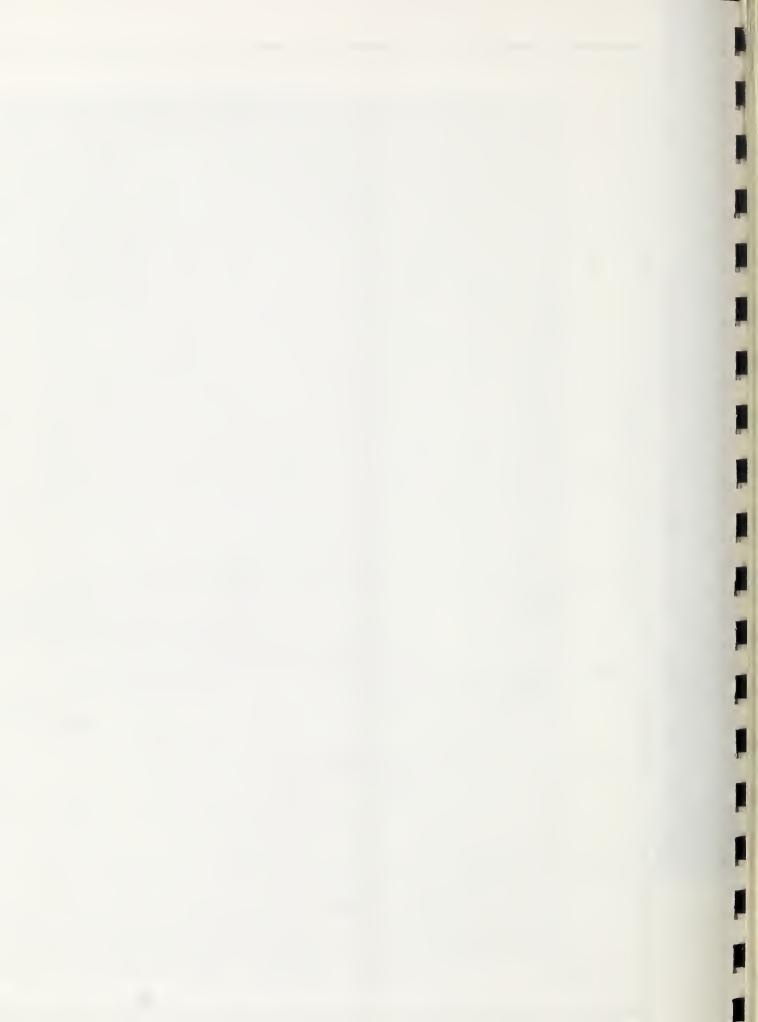


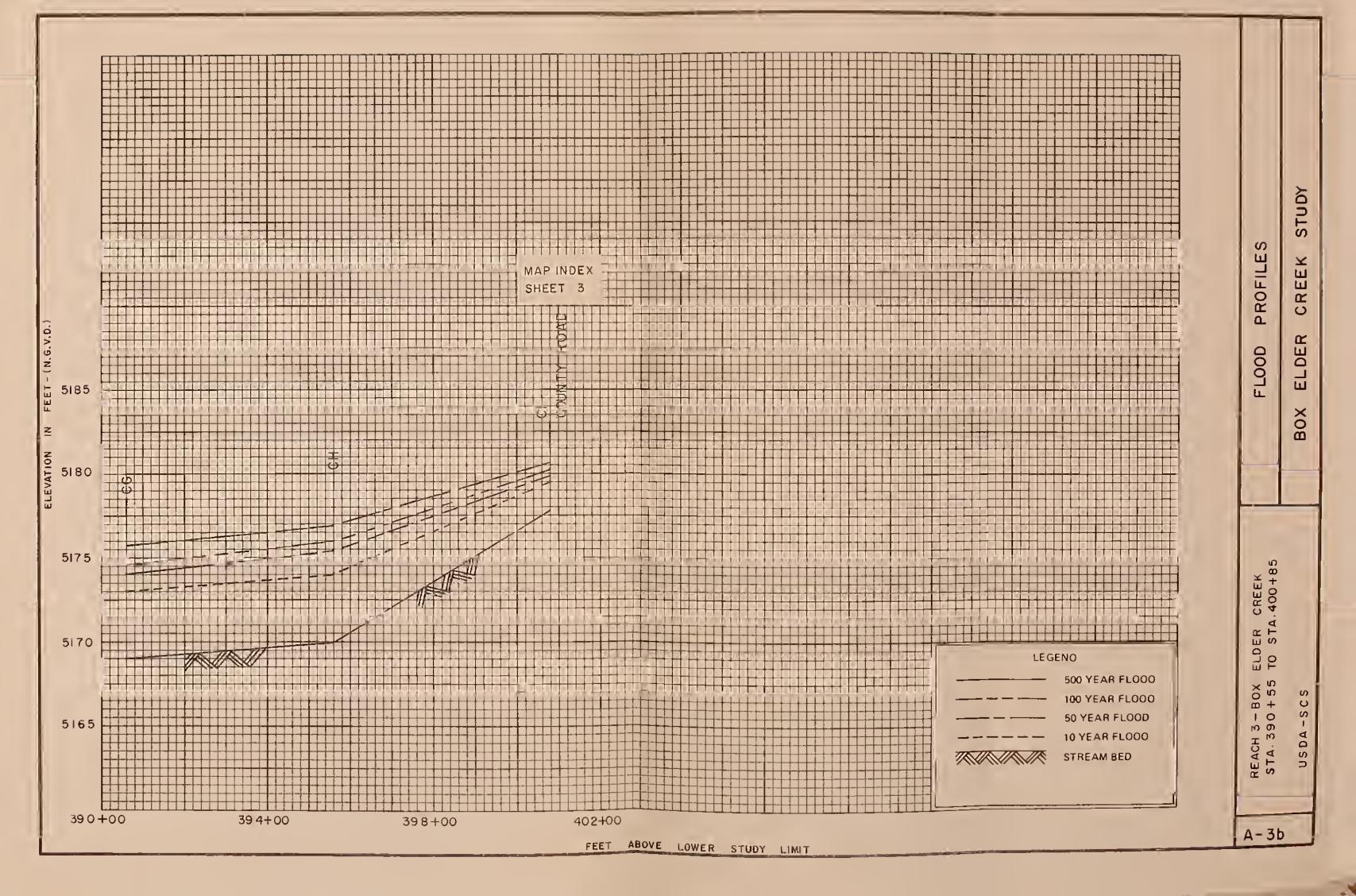




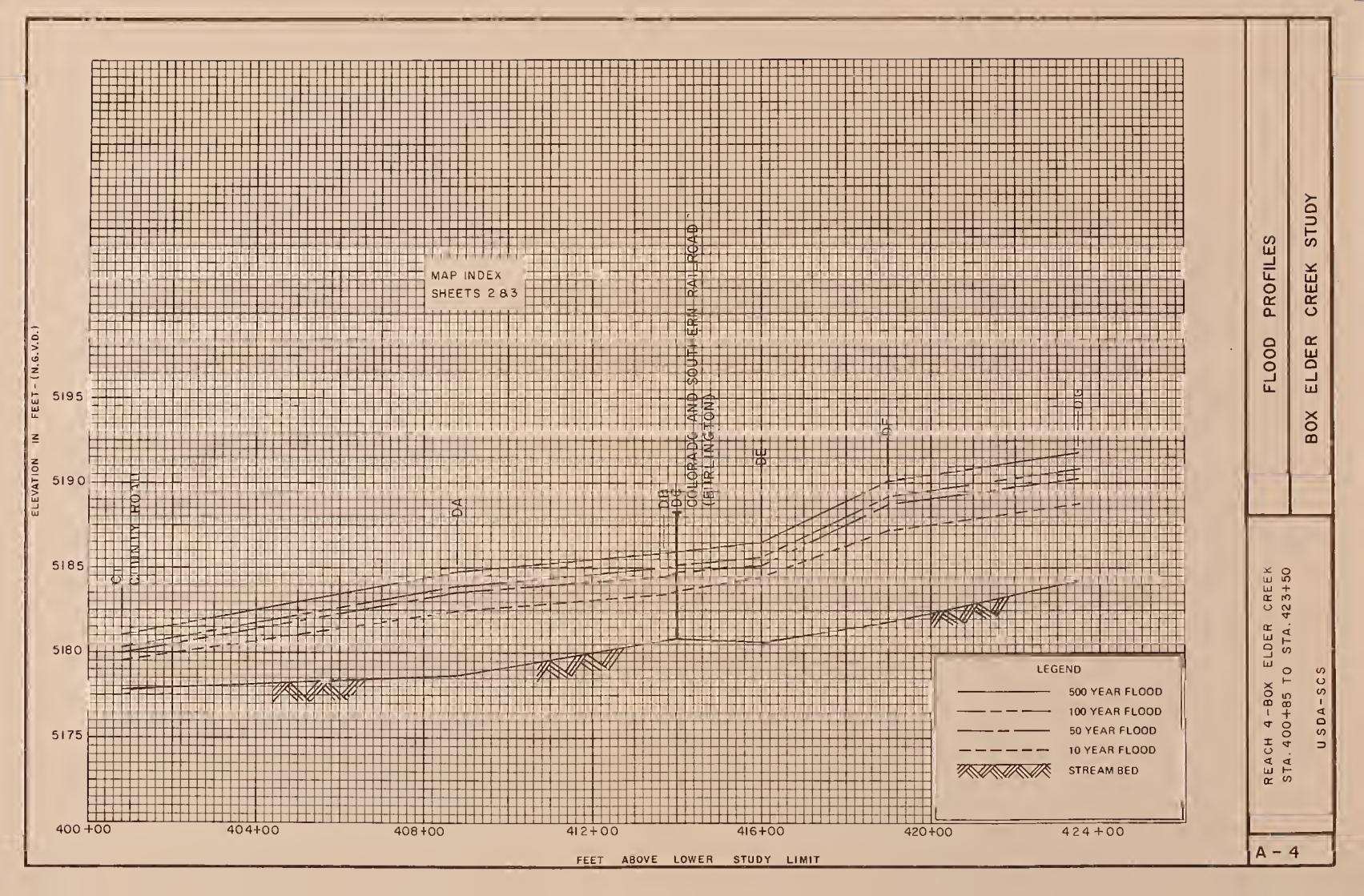


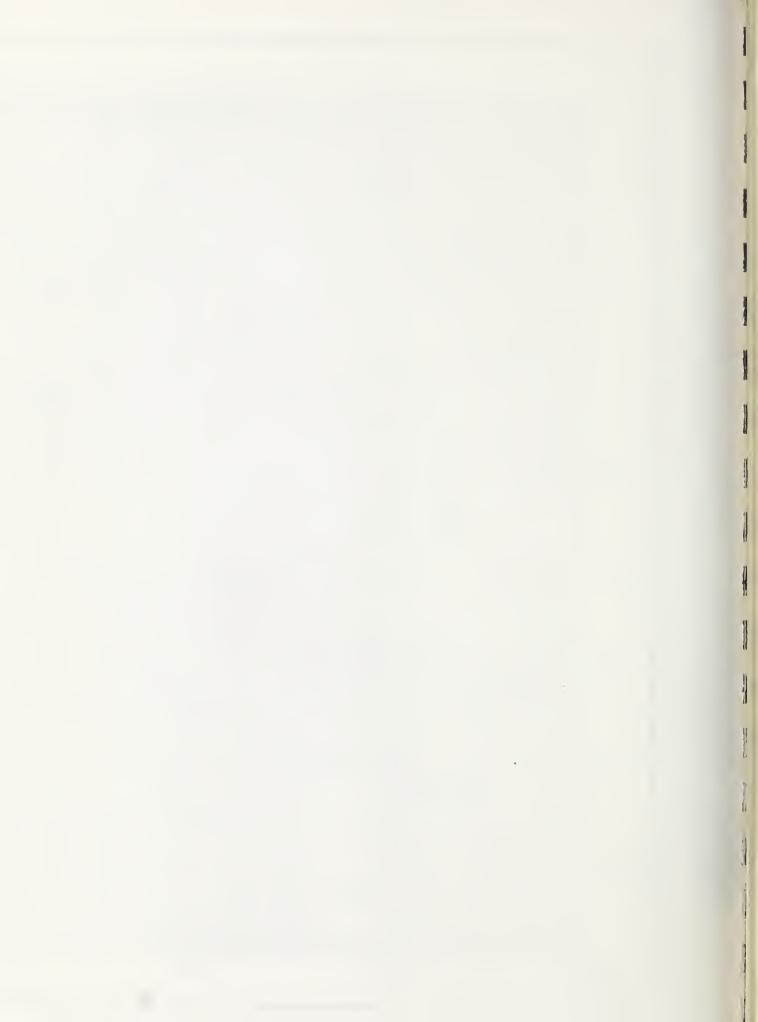


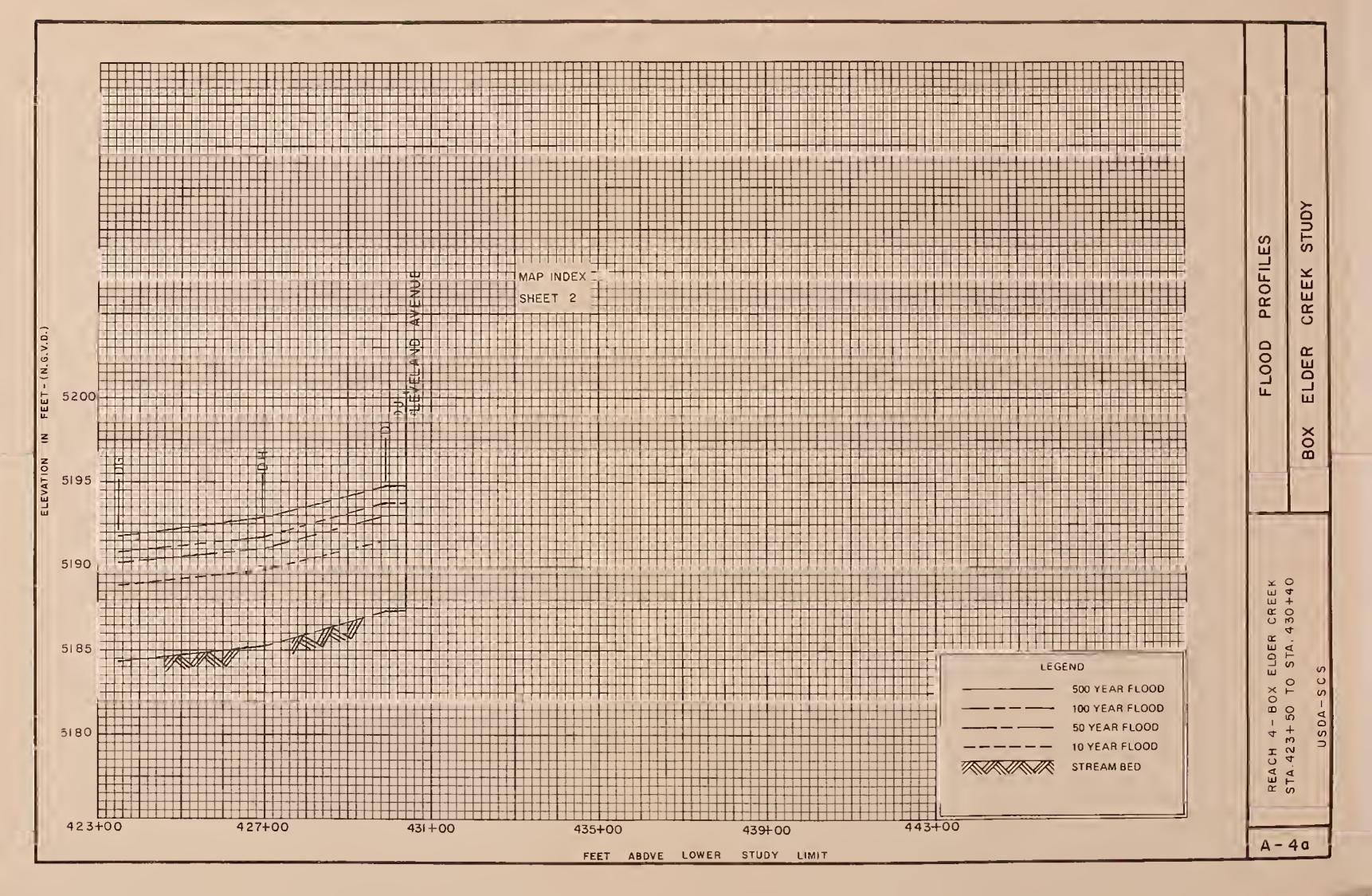




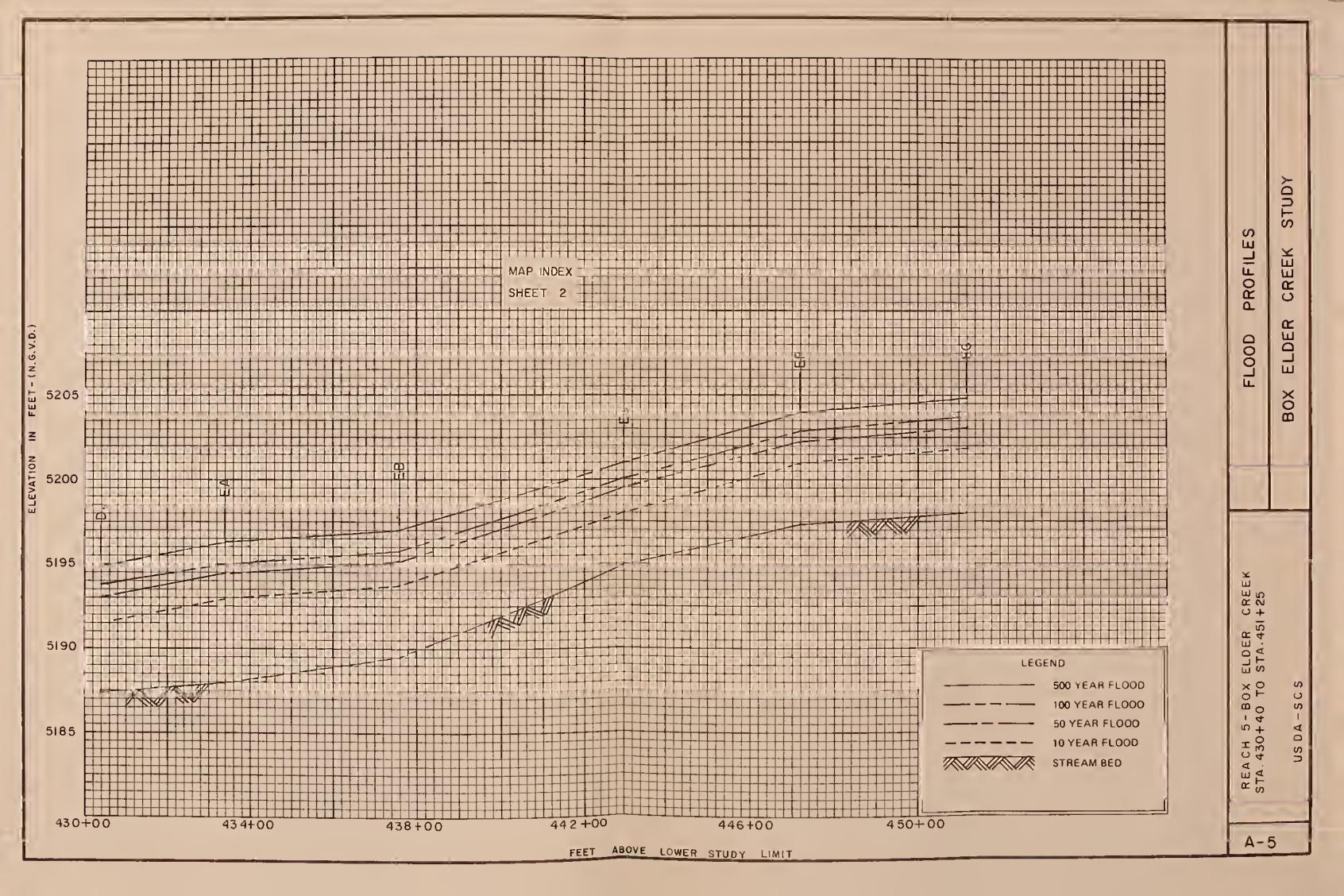




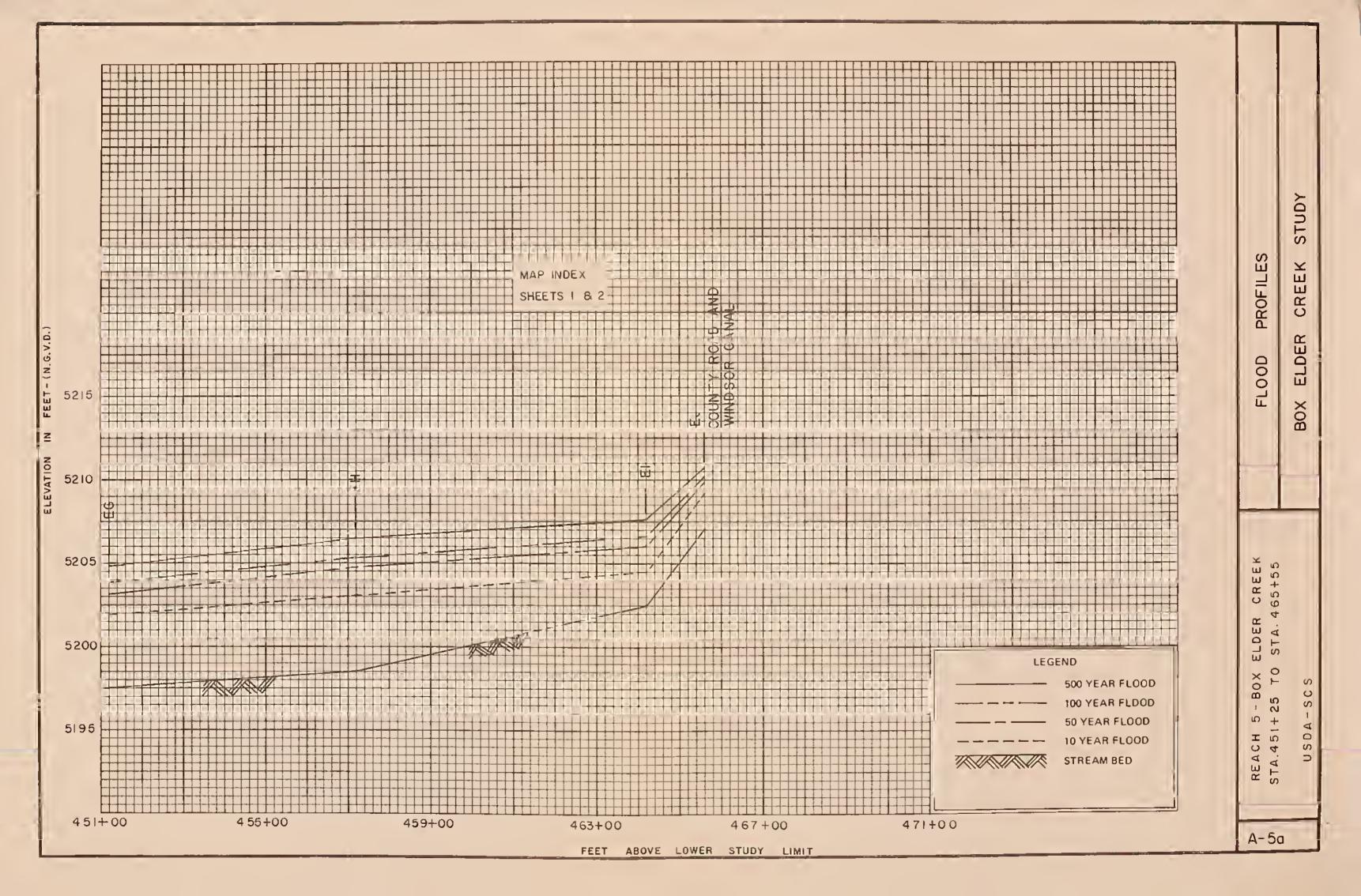




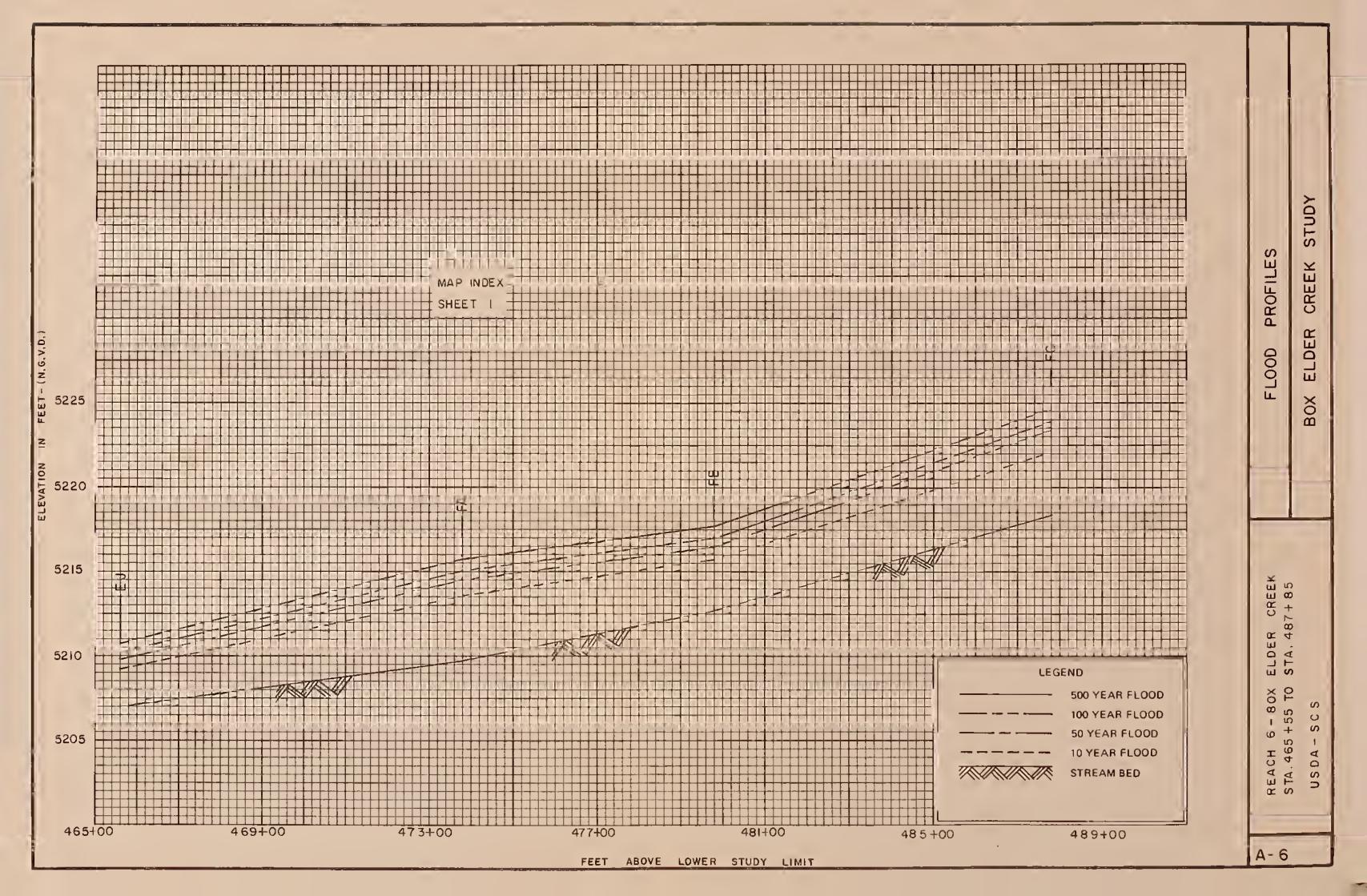




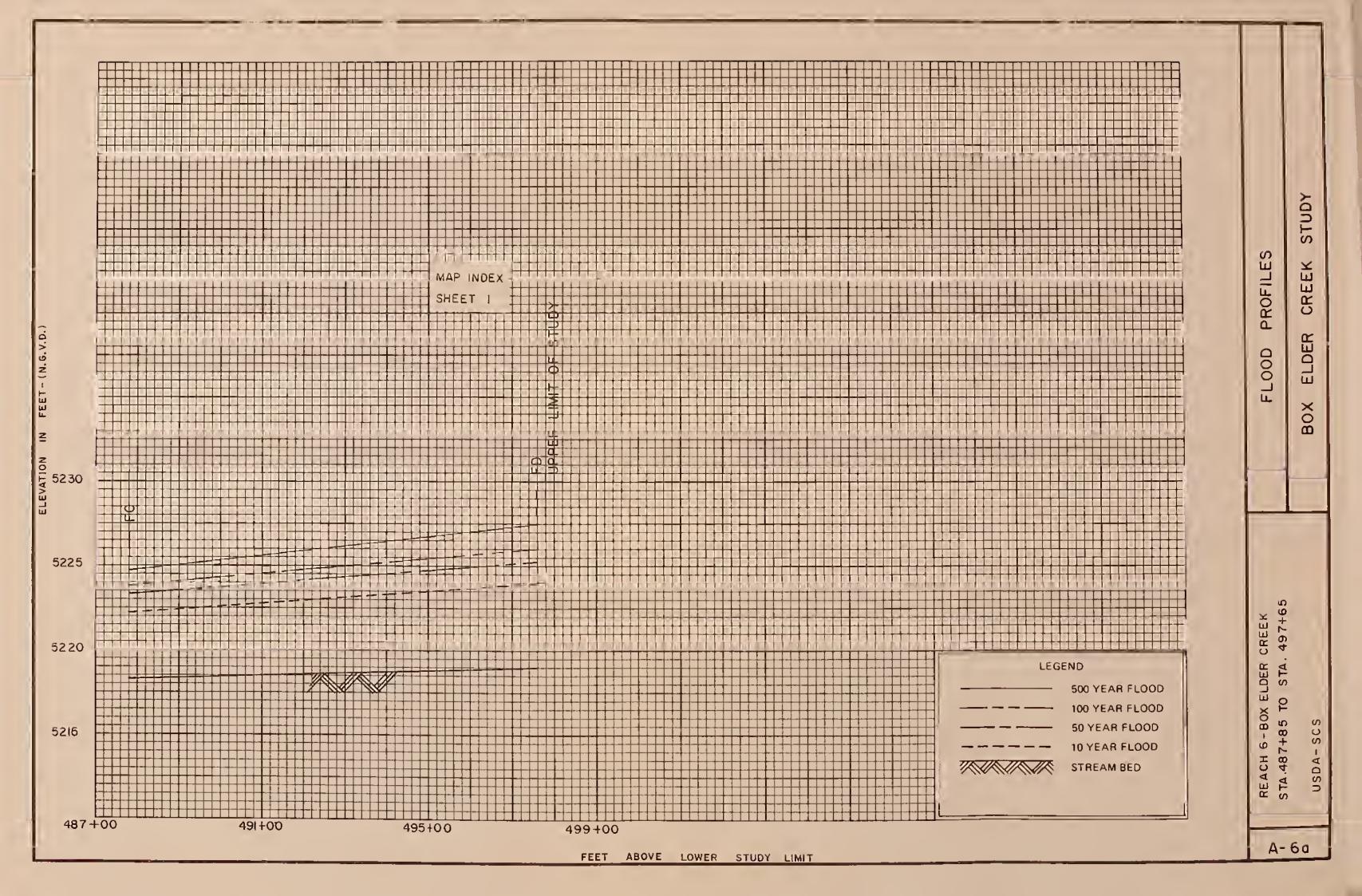




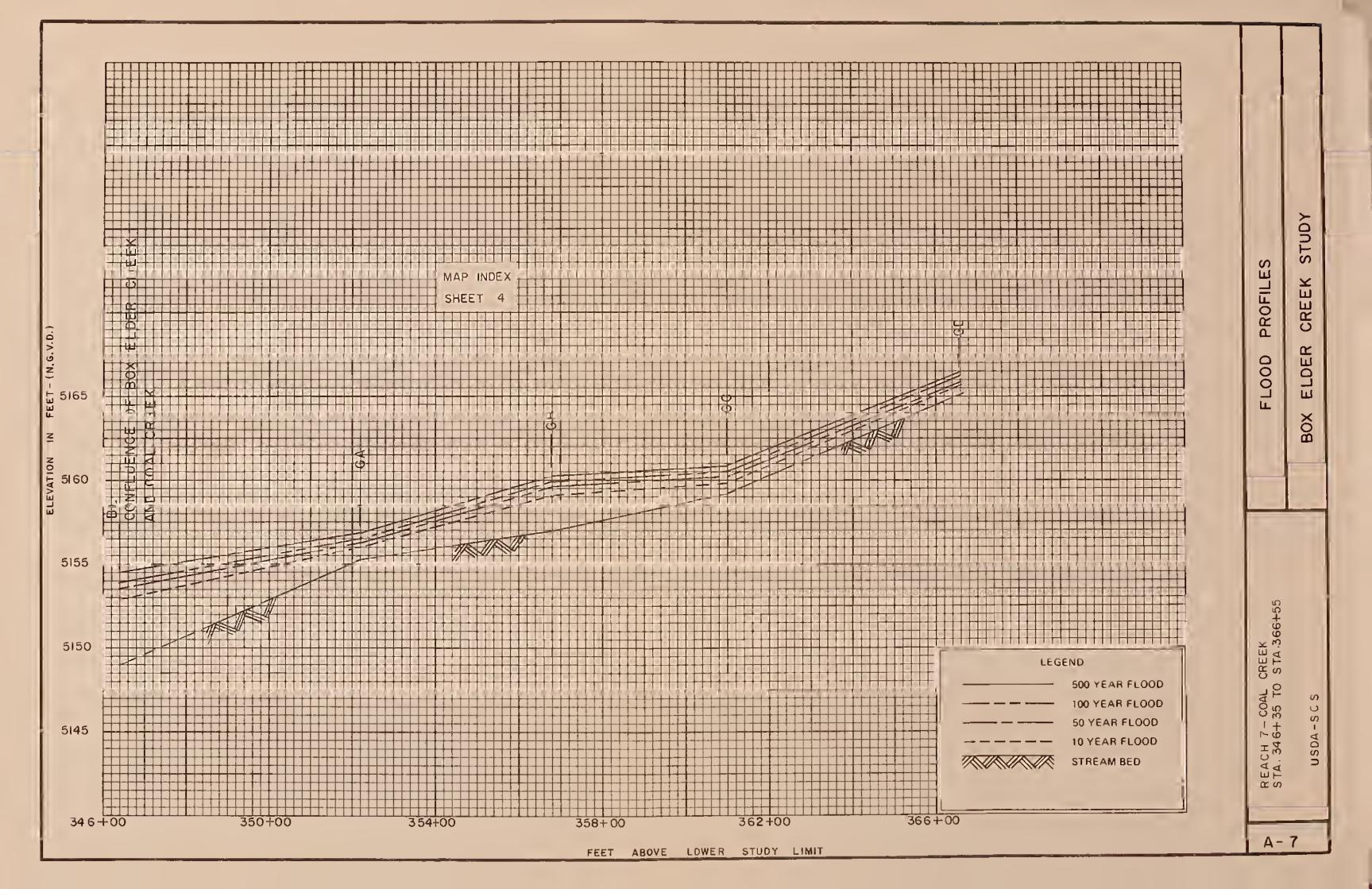


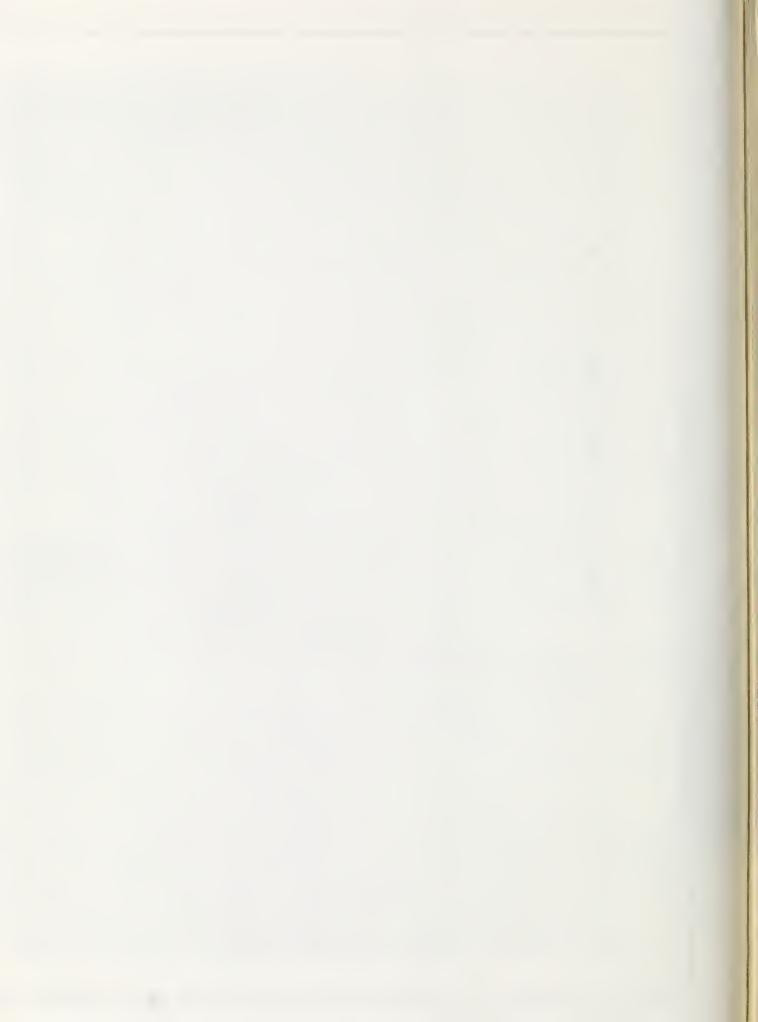


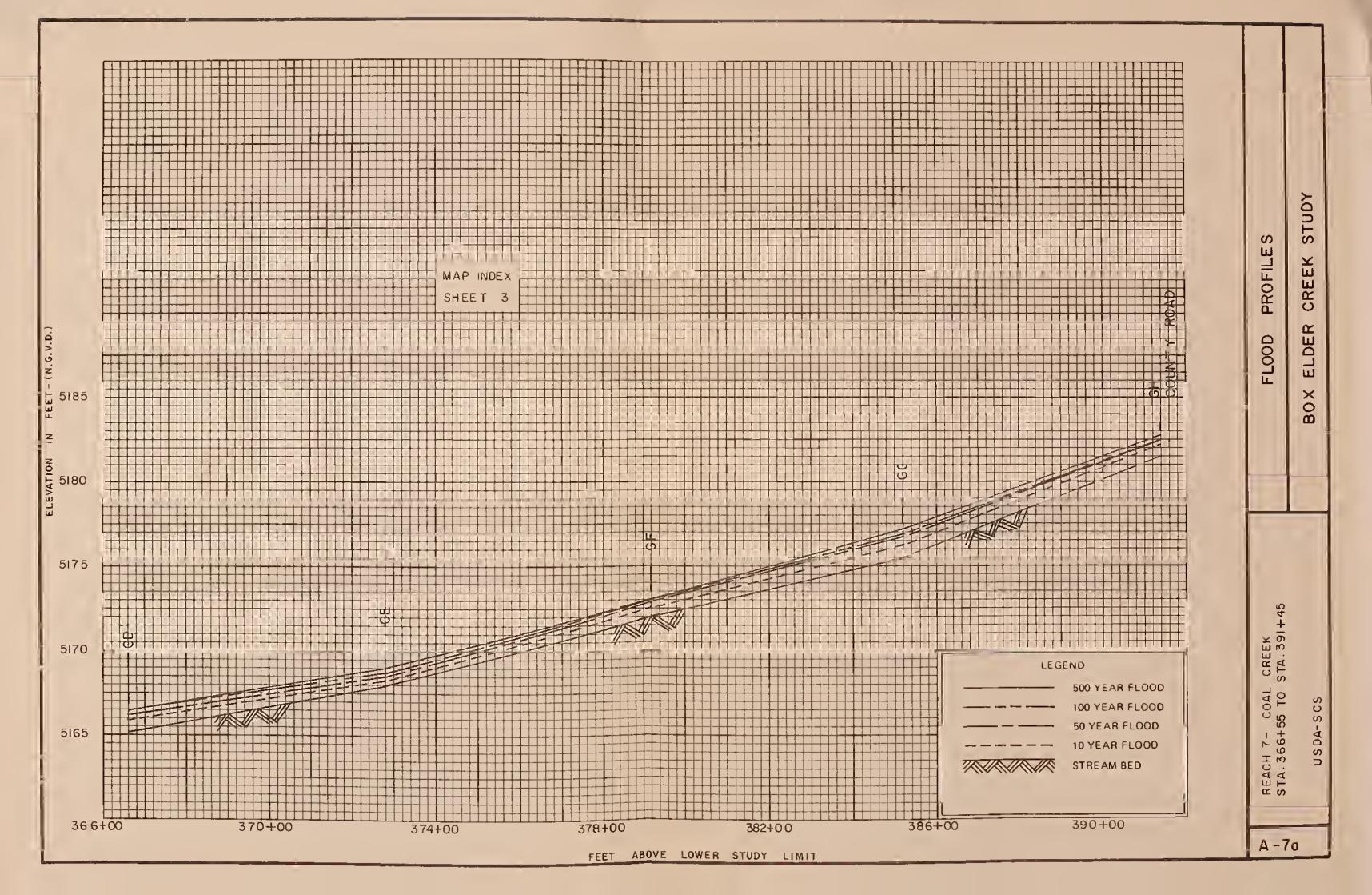


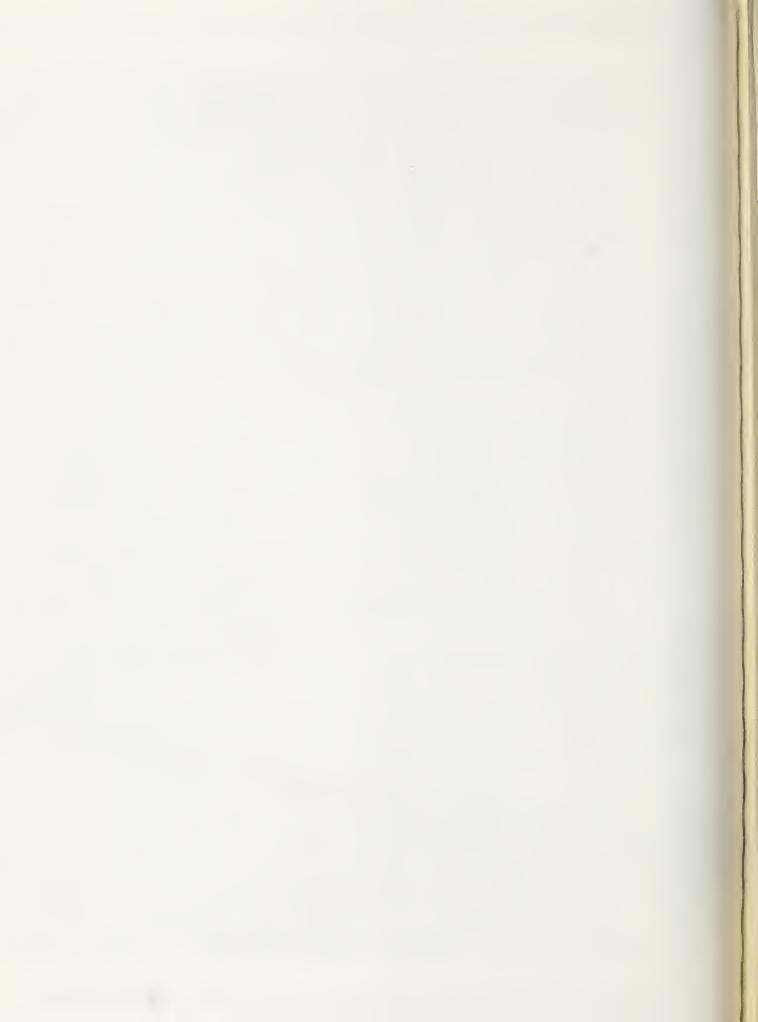


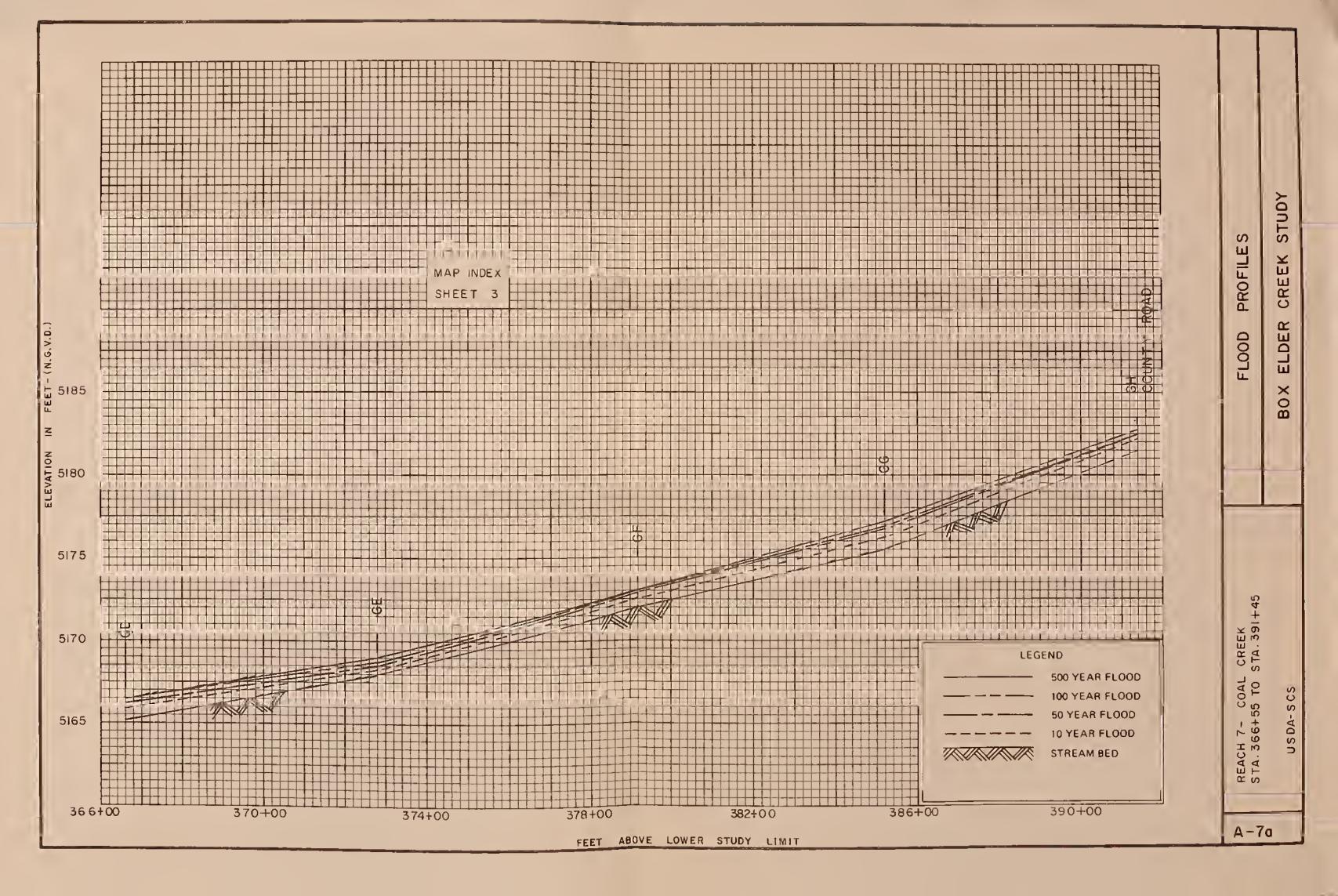


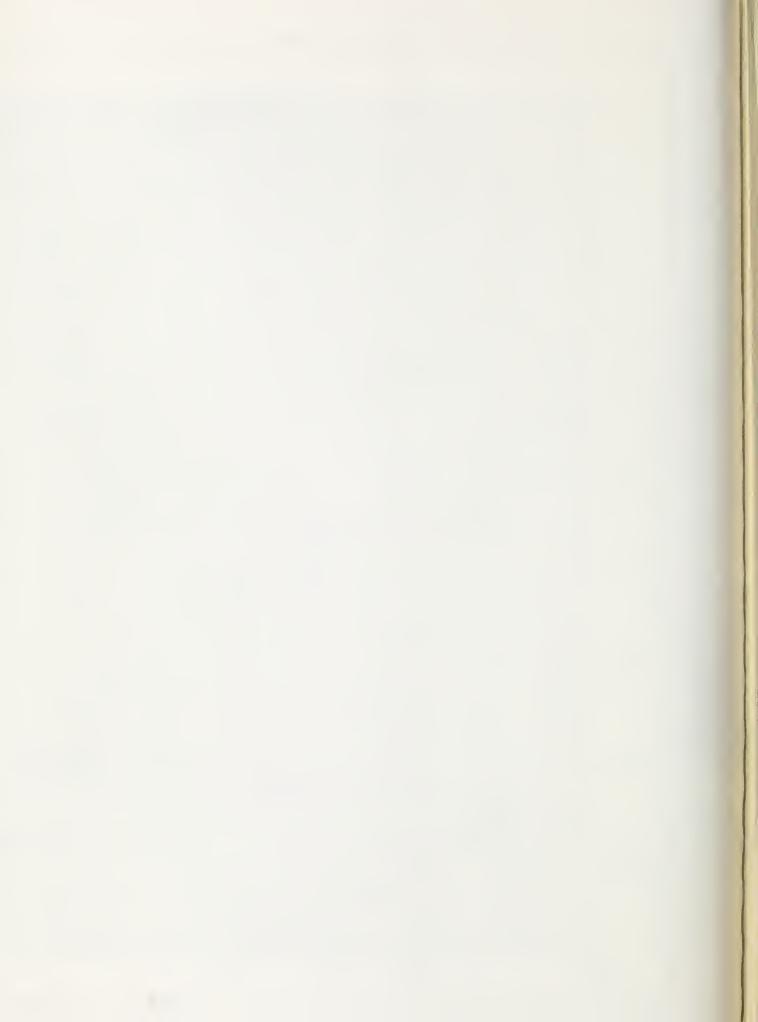


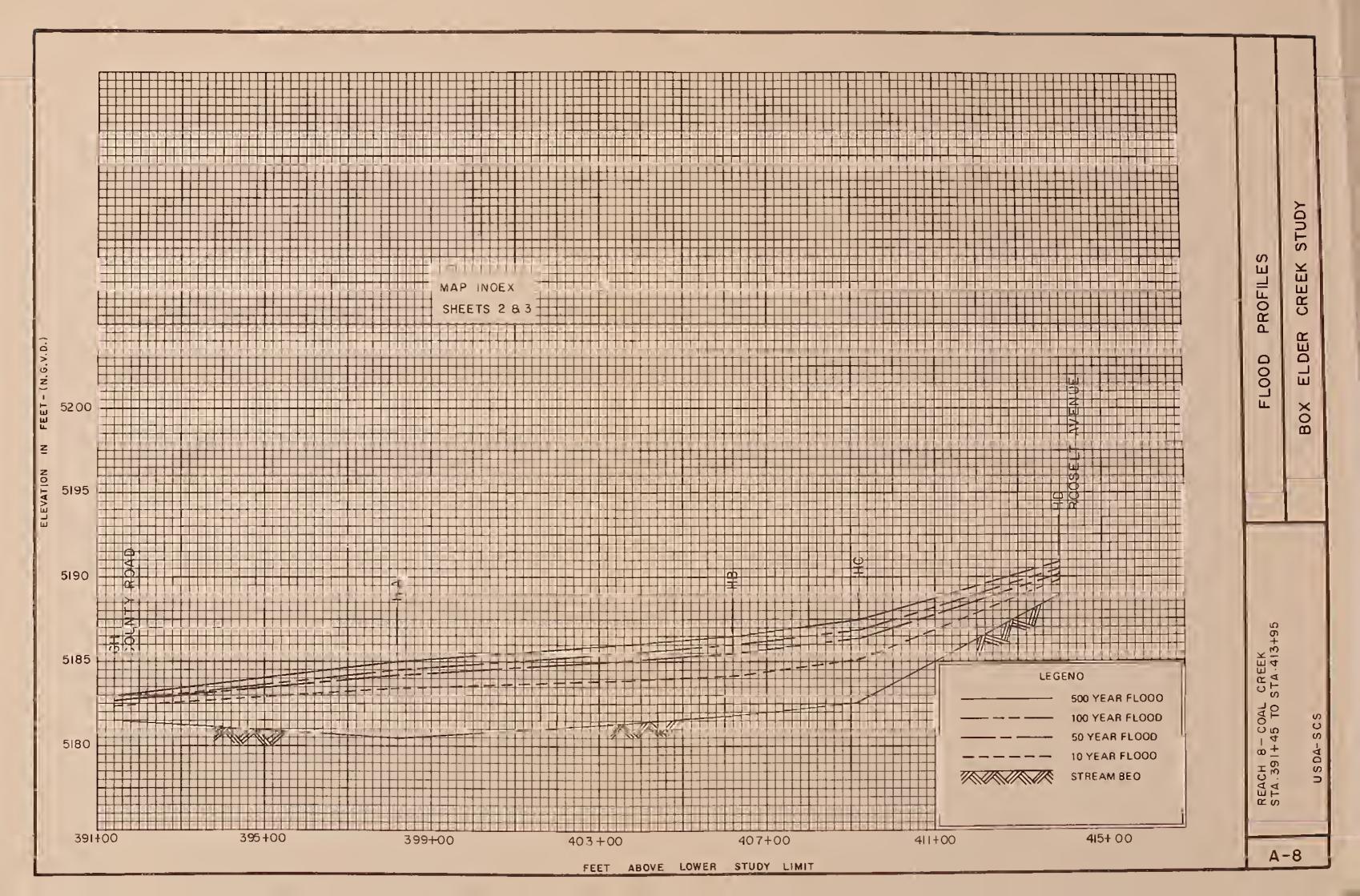




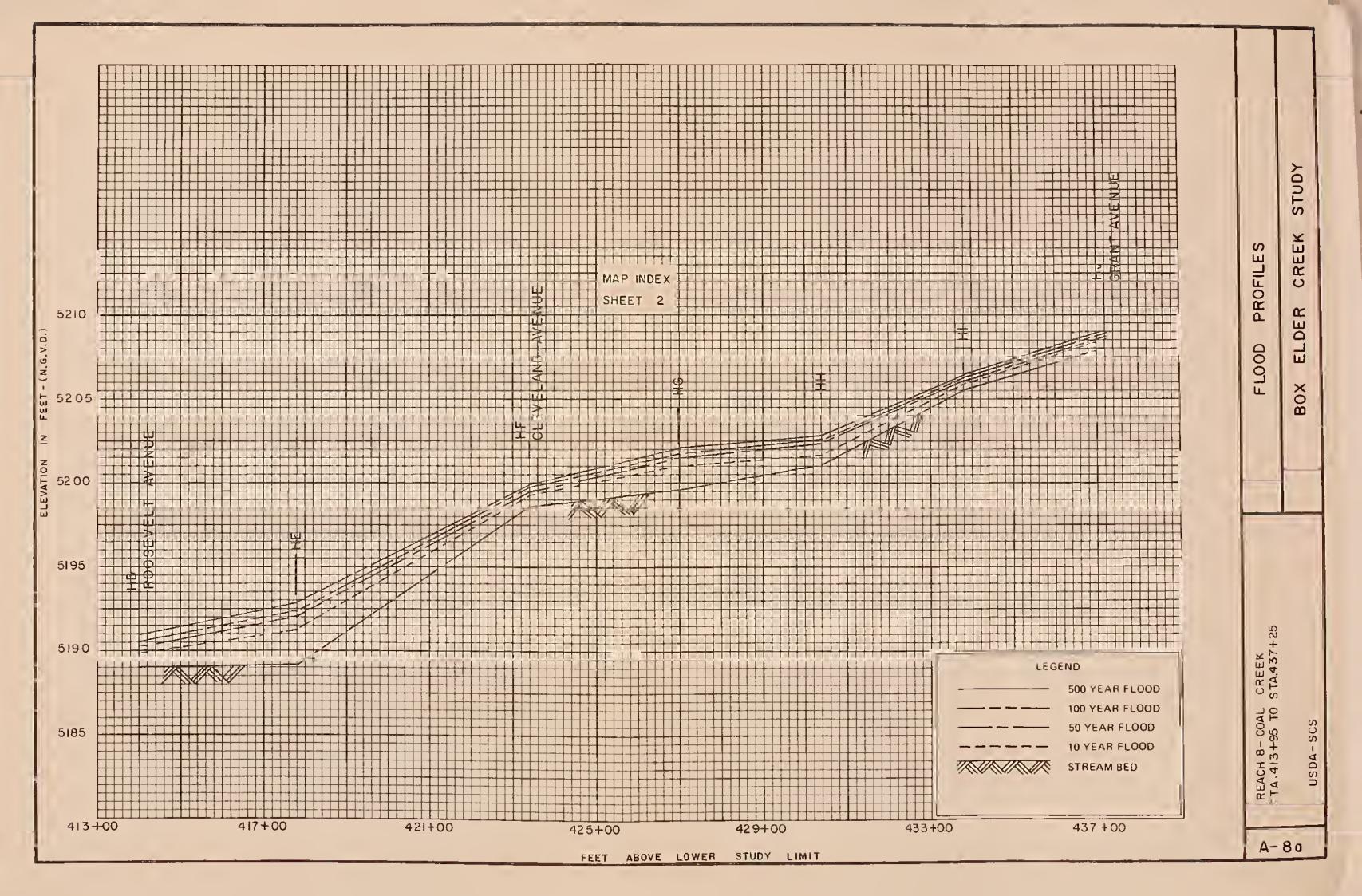




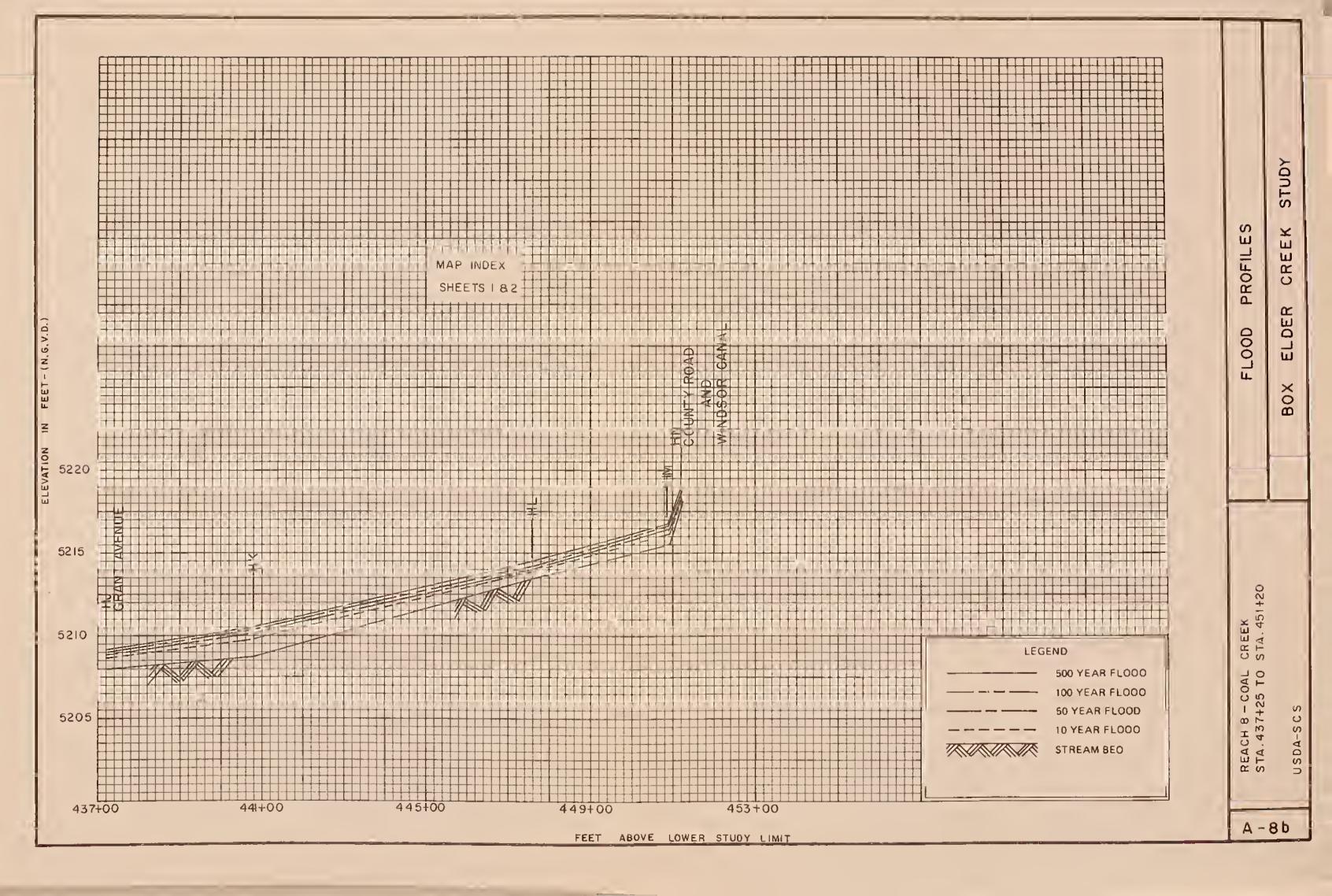




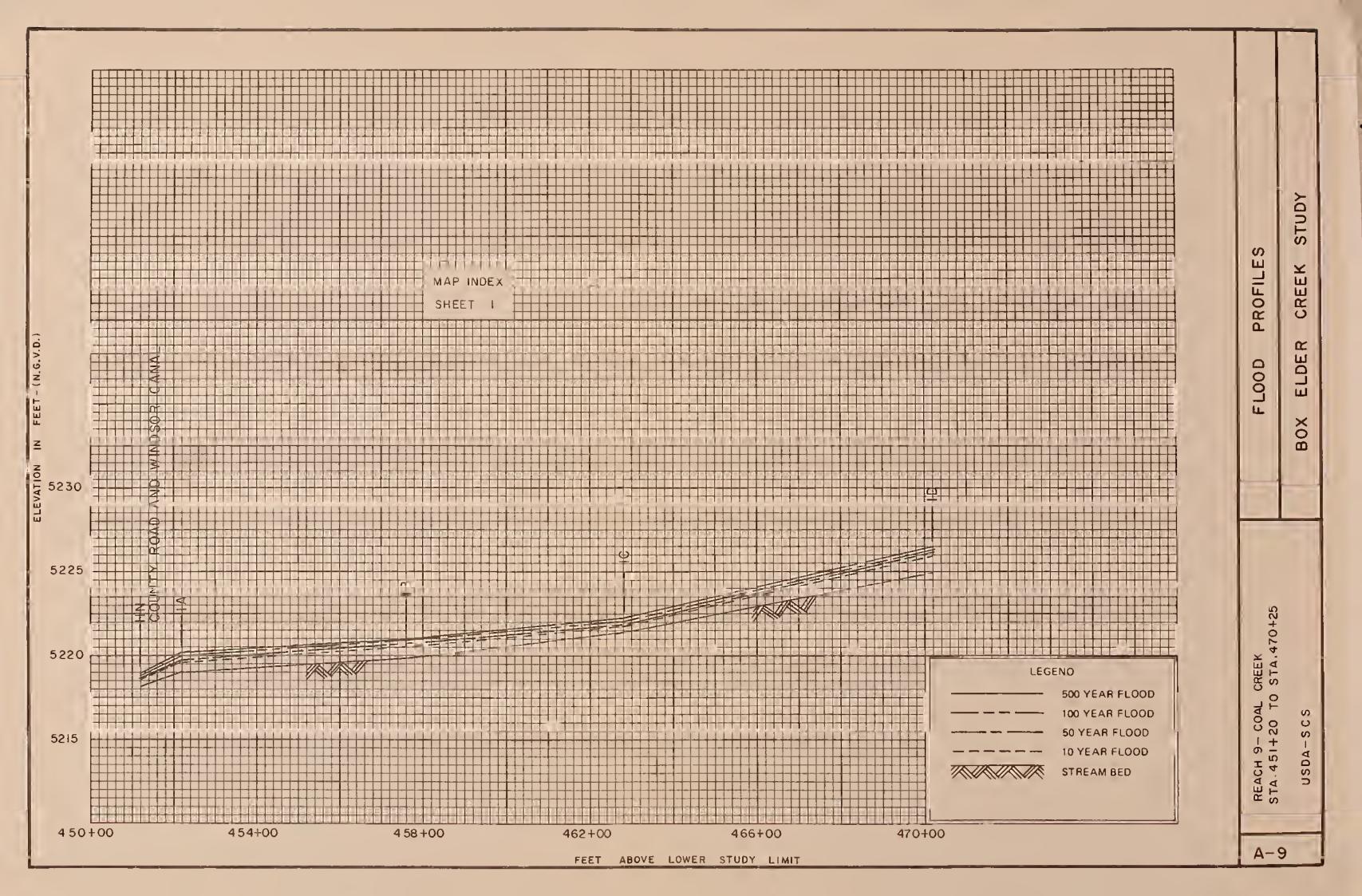




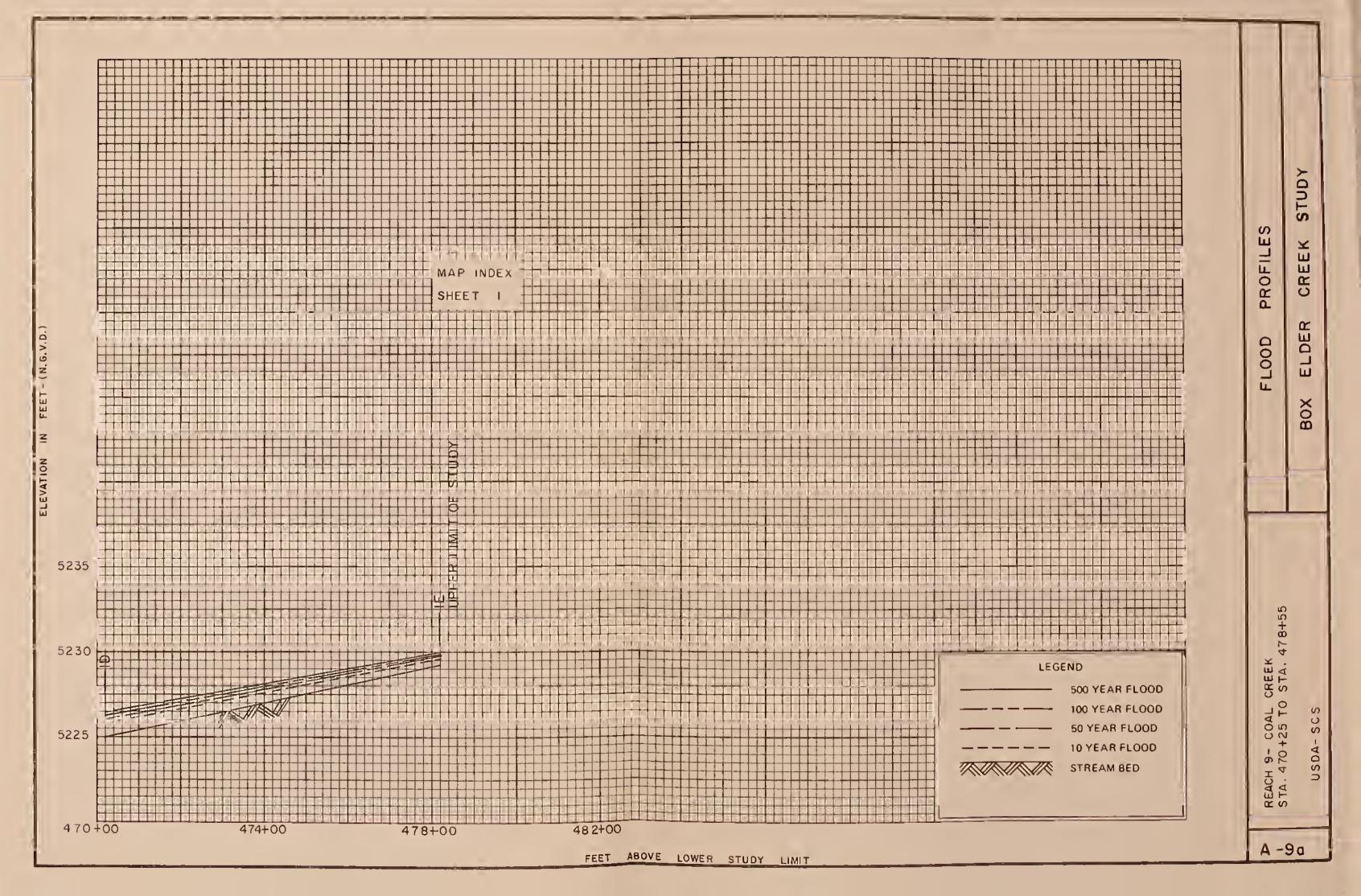




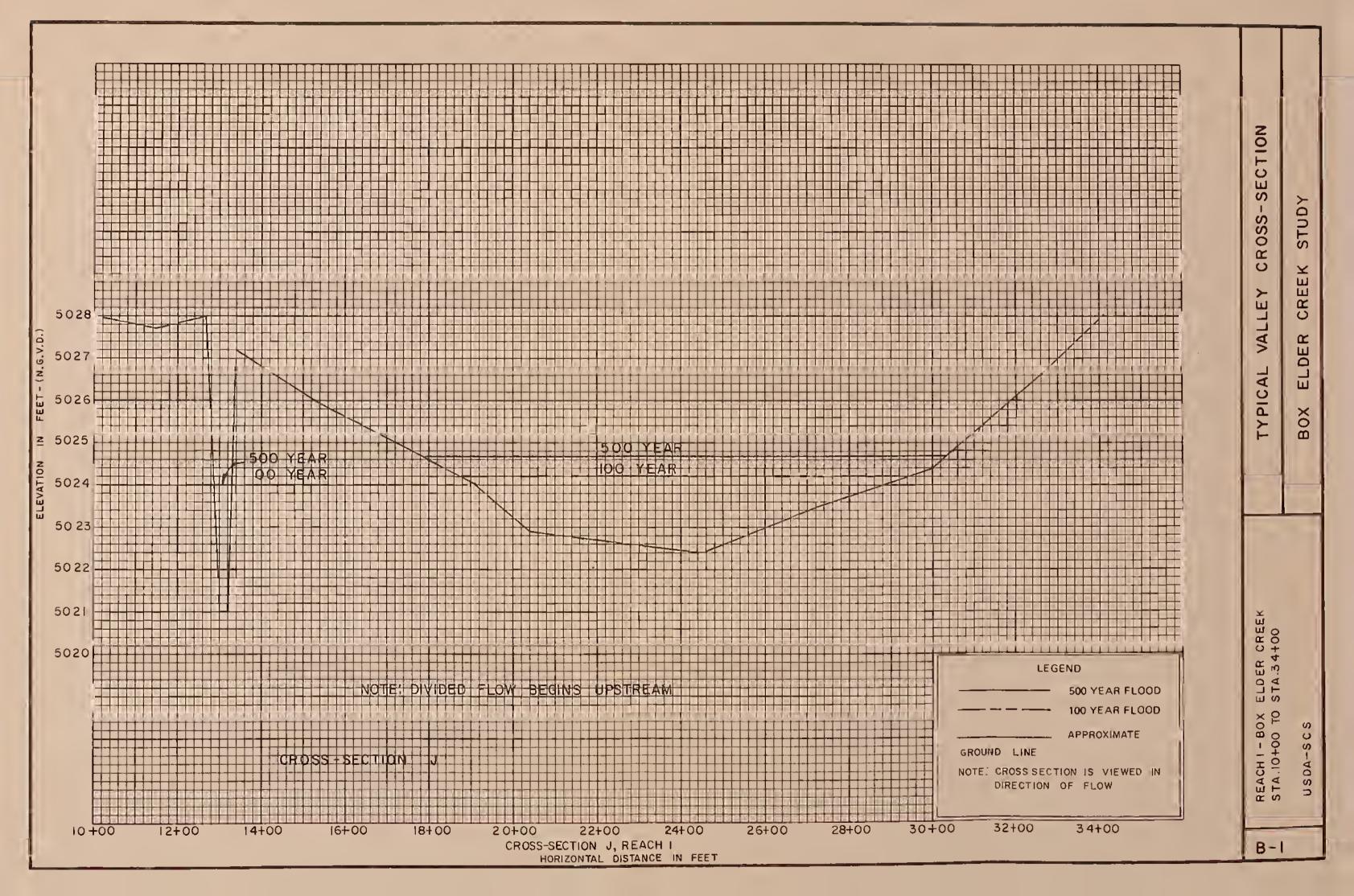




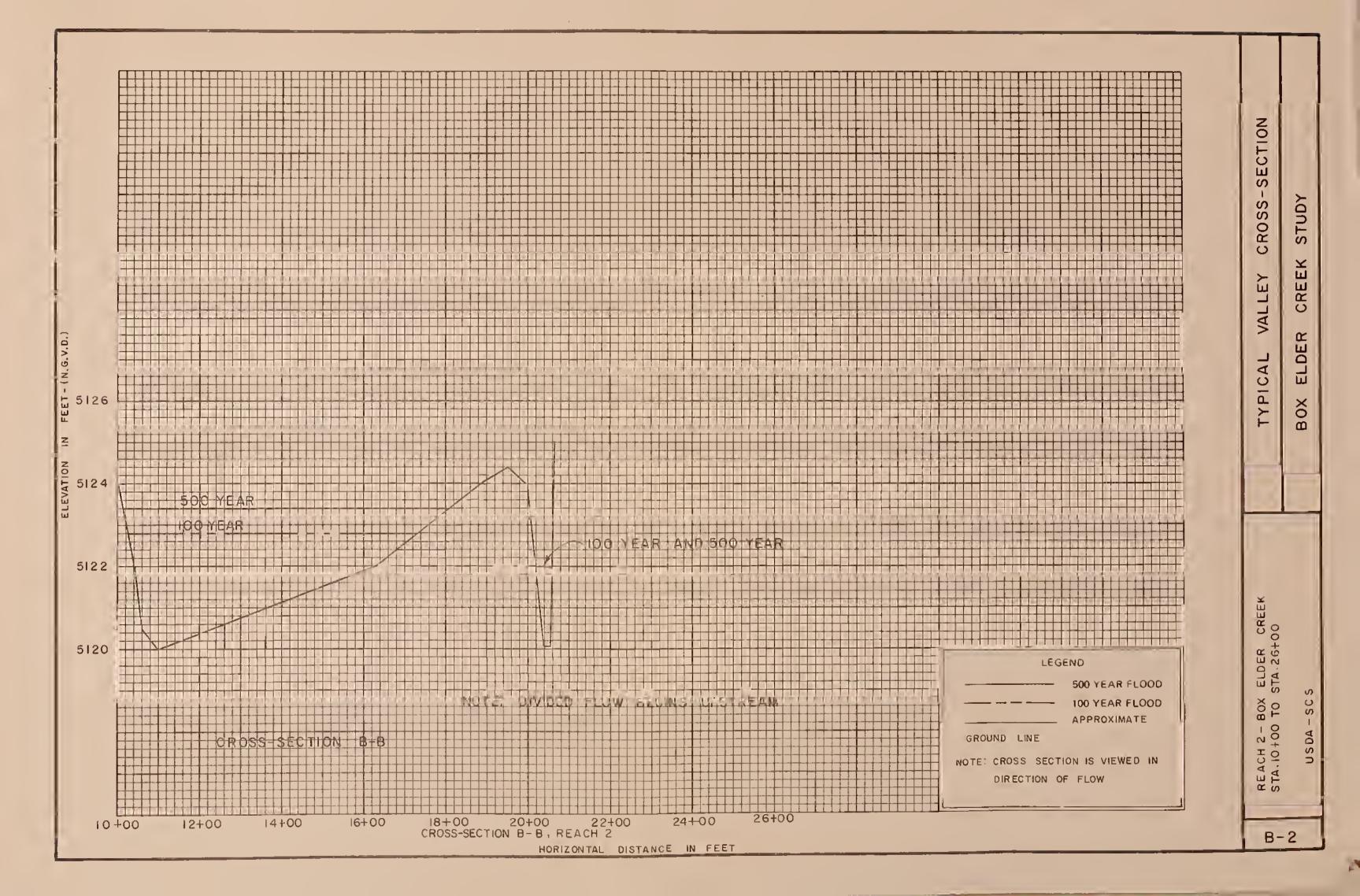




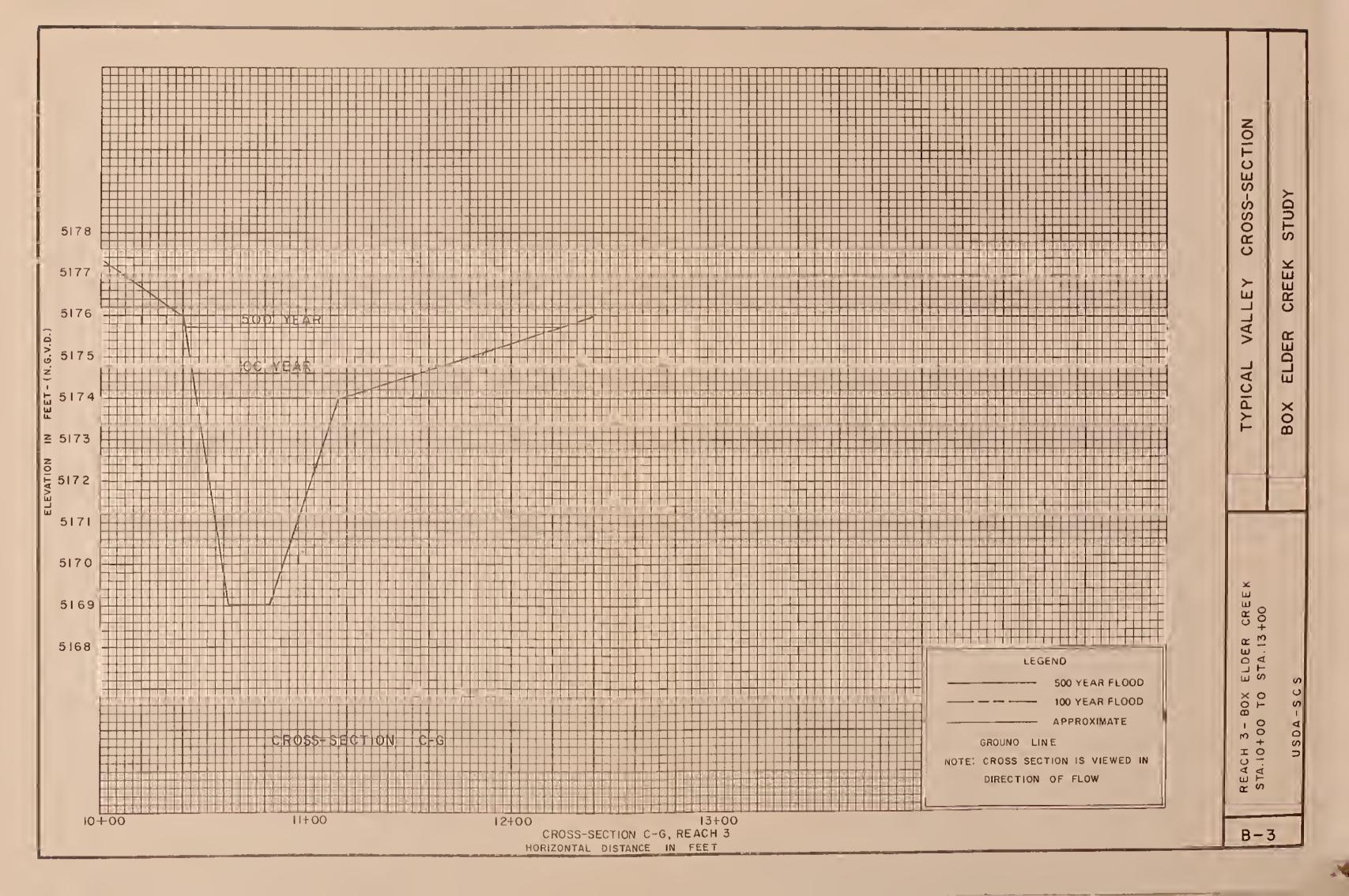




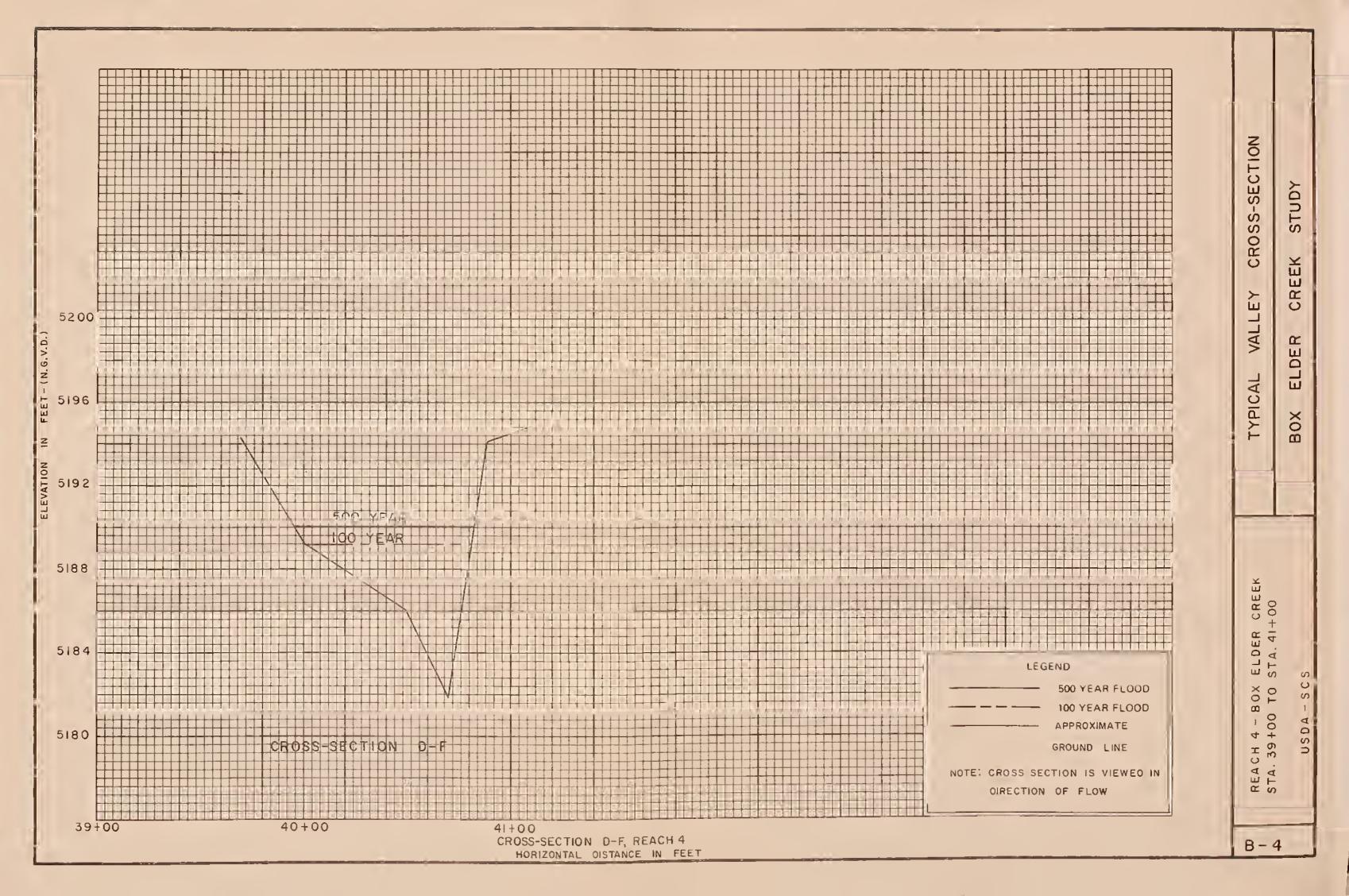




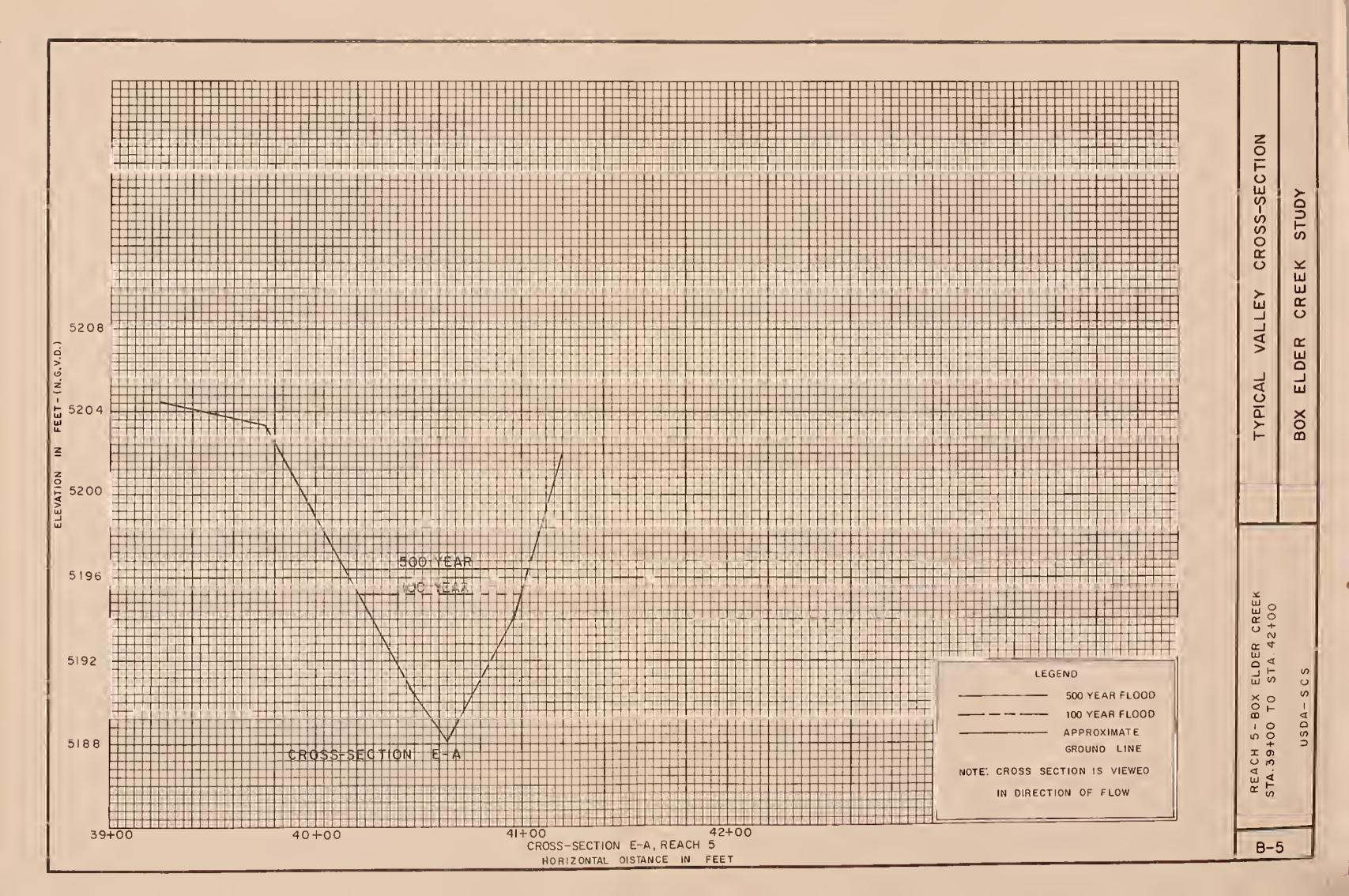




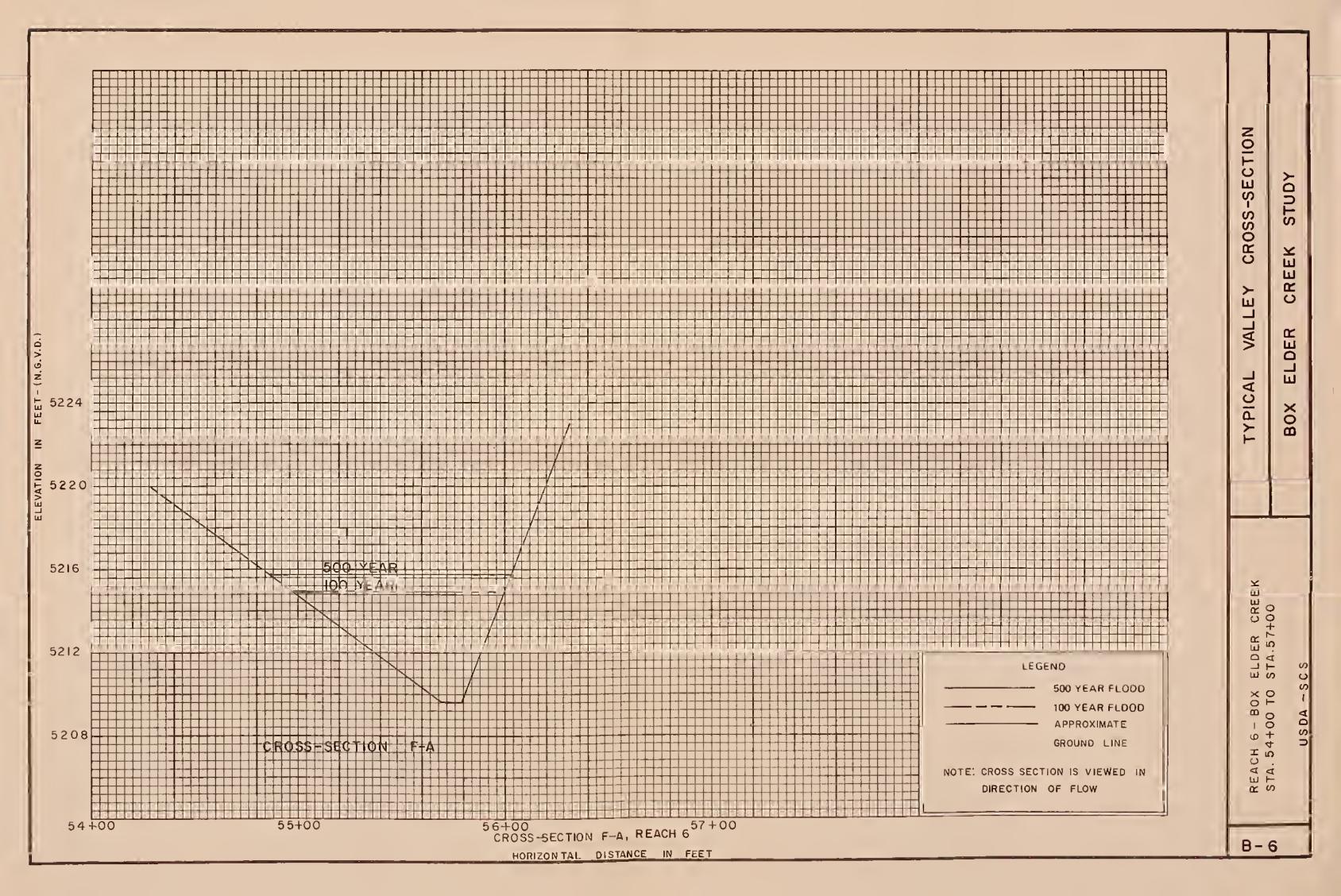




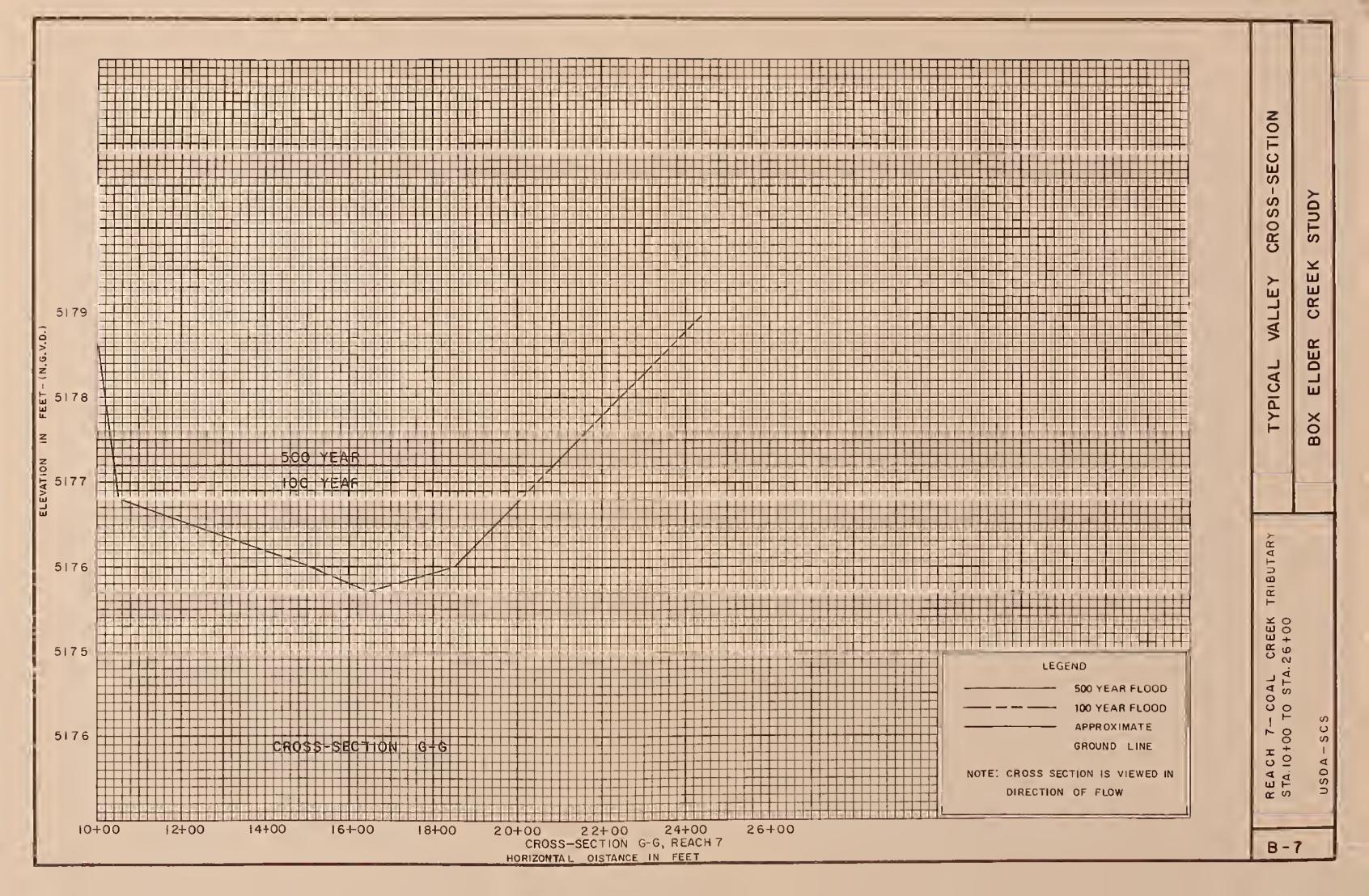




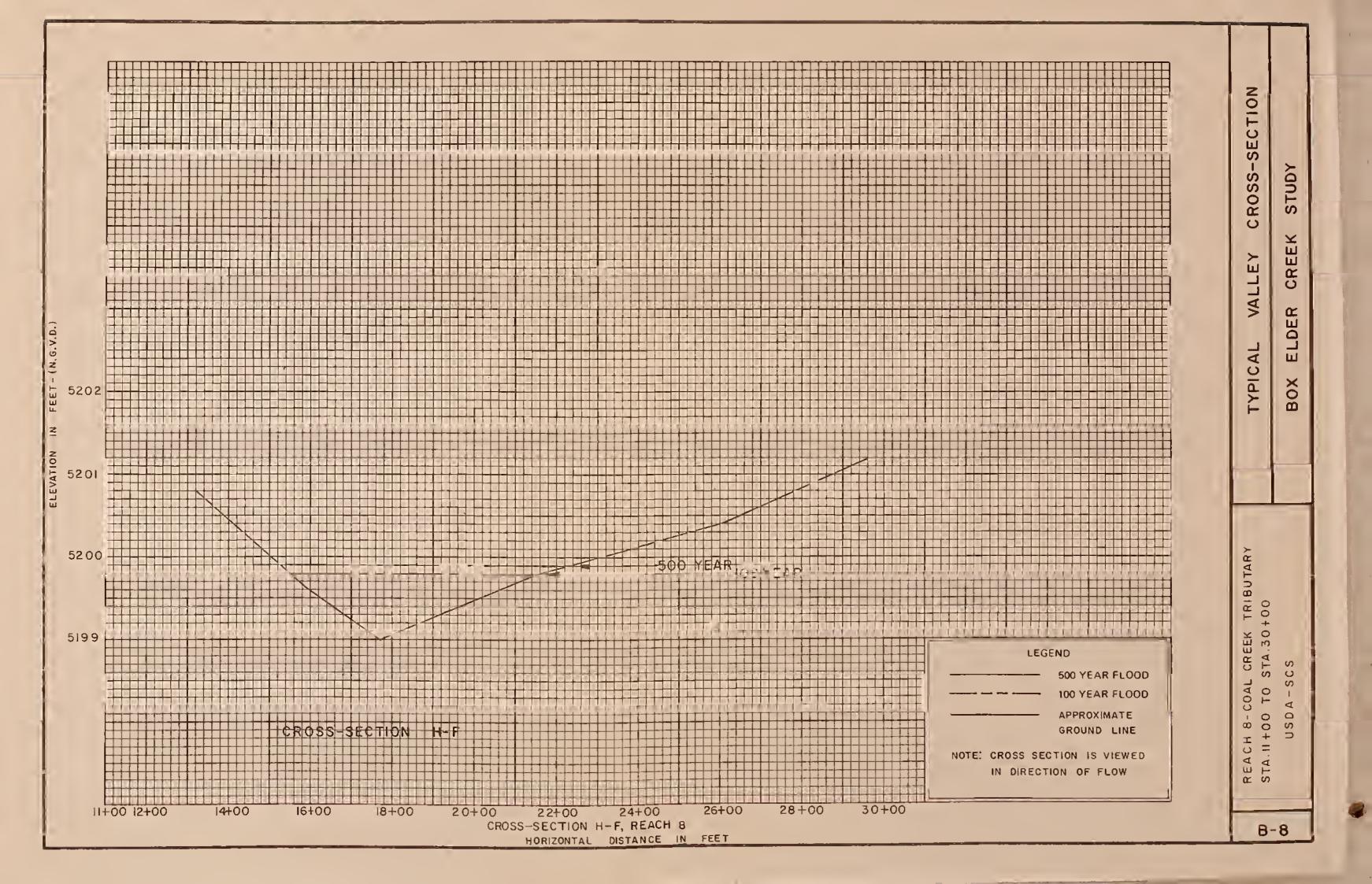


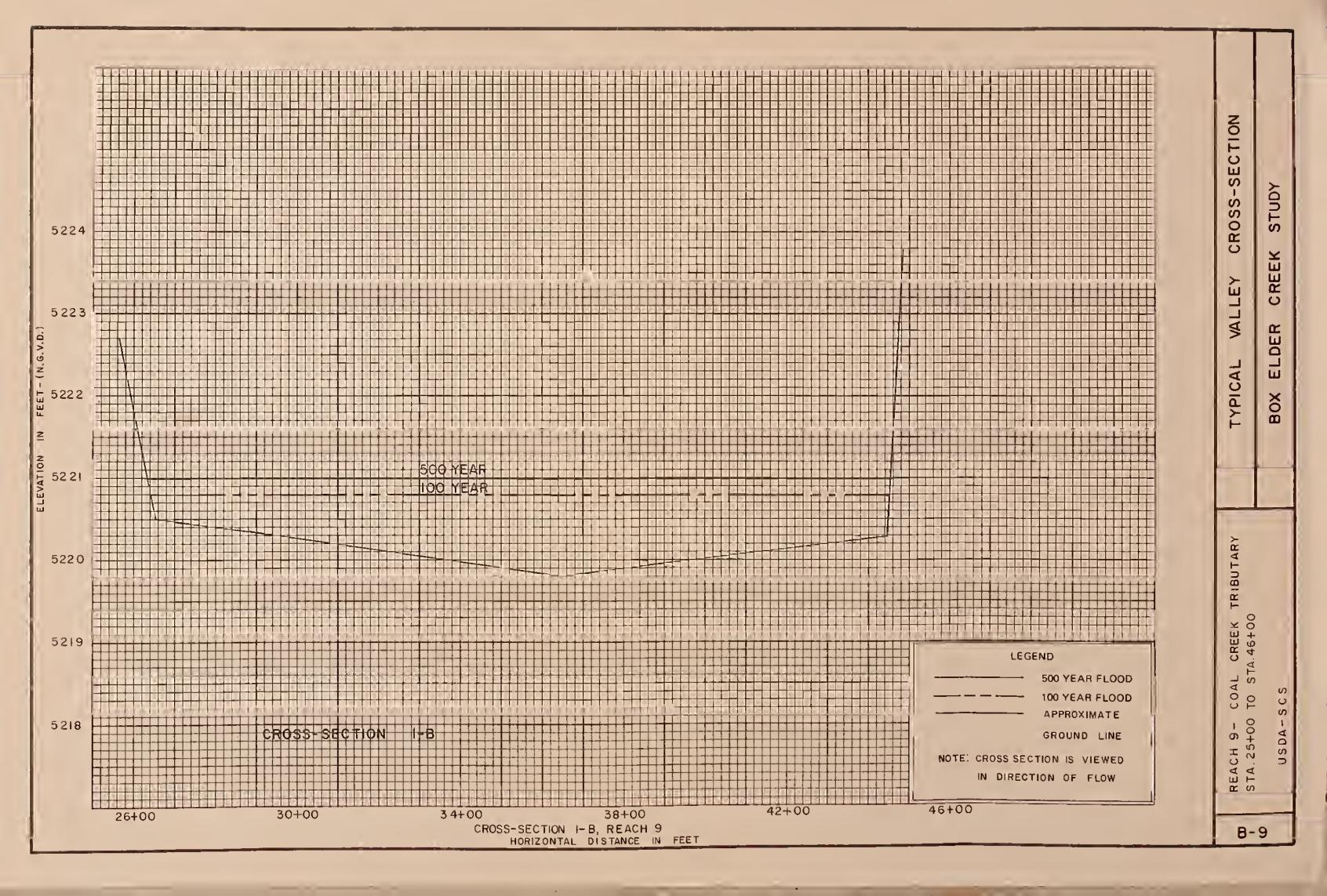












Vertical	500-Year Flood	4996.2	4996.8	4997.0	4998.8	5004.1	5008.1 5790	5013.2	5020.4 5790	5022.9
Crest-Elevation Feet National Geodetic V Datum, and Peak Discharge c.f.s.	: 100-Year : Flood	4995.7 3690	4996 .1 3690	4996.3	4998.6 3690	5003.8	5007.7 3690	5012.9 3690	5020.1 3690	5022.4
ation Feet Nat	: 50-Year : Flood	4995.3 2630	4995.7 2630	, 4995.8 2630	4998.5 2630	5003.6 2630	5007.5 2630	5012.7 2630	5019.9 2630	5022.1 2630
: Crest-Elev	: 10-Year : Flood	4994.4	4994.7	4994.7	4998.2	5003.2	5007.0	5012.2	5019.4	5021.2
: Stream Bed	Elevation (ft.) N.G.V.D.	4991.8	4990.5	4992.0	4998.0	5002.4	5006.0	5011.1	5018.1	5019.2
	Identification	Begin Reach l Divided Flow	Divided Flow	Divided Flow	Divided Flow	Divided Flow	Divided Flow	Divided Flow	Divided Flow	Divided Flow
Stationing :	Study Limit : (feet) :	0+00	6+85	13+30	20+55	26+55	36+15	45+80	52-90	59+70
Cross :	Desig- :	A-West	R-West	C-West	D-West	E-West	F-West	G-West	H-West	I-West

plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood road crossings, upstream diversions, etc.

Cross :	Stationing			•••	Crest-Ele	vation Fe	et Nat	iona	Crest-Elevation Feet National Geodetic Vertical	Ver	tical
Section :	From Lower	••		: Stream Bed :	I	atum, and	Peak	Disch	Datum, and Peak Discharge c.f.s.	s °	
Desig- :	Study Limit	••		: Elevation :	10-Year	: 50-Year	ear		100-Year		500-Year
nation :	(feet)	••	Identification	: (ft.) N.G.V.D.:	Flood	: Flood	pod	••	Flood	••	Flood
J-West	68+50		Nivided Flow	5022.4	5023.2 990	5023.9	6.1	2 , (, ,	5024.2 3690		5024.7 5790
K-West	75+30		Divided Flow	5026.0	5027.7 990	5028.3	.3		5028.6 3690		5029 .0 5790
L-West 2/	85+30			5031.2	5032.2	5032.8 2780	8.	<b>u</b> , (, )	5033.1 3840		5033.5 5940

This is the upstream end of a divided flow reach. Water surface elevation may be slightly different than shown on the main channel at cross section L. 2/

Table page 2

tical	500-Year Flood	4984.8	4987.5	4990.5	4992.9	4997.8	5002.6	5006.3	5017.9	5022.5
etic Ver	ar :									
Crest-Elevation Feet National Geodetic Vertical Datum, and Peak Discharge c.f.s.	100-Year Flood	4984.8	4987.5	4990.5	4992.9	4997.8 150	5002.6	5006.3	5017.9	5022.5
Nation sak Dis										
on Feet	101	4984.8	4987.5	4990.5	4992.9	4997.8	5002.6	5006.3	5017.9	5022.5
levatio Datum,										
Crest-	10-Year Flood	4984.8	4987.5	4990.5	4992.9	4997.8	4002.6	5006.3	5017.9	5022.5
	n V D									
Stream Bed	Elevation (ft.) N.G.V.D	4982.0	4984.6	4989.2	4993.0	0.9664	5000.0	5005.3	5016.0	5020.5
•••••										
	Identification	Begin Reach l Divided Flow	Divided Flow	Divided Flow	Divided Flow	Divided Flow	Divided Flow	Divided Flow	County Road Divided Flow	Divided Flow
ing	imit									
Stationing From Lower	Study Limit (feet)	00+0	7+80	15+20	36+45	36+75	47+15	55+85	64+25	72+45
	••									
Cross	Desig- nation	A	23	0	Q	E	ŭ.	9	н	

plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood road crossings, upstream diversions, etc.

Cross	. Crationing	•	•	Crest-Flowst	fon Foot Natio	Creet-Flowation Root National Condetic Vertica	prticol
Section	: From Lower	• ••	Stream Bed :	Datum,	m, and Peak Di	and Peak Discharge c.f.s.	stricai
Desig-	: Study Limit		Elevation :	10-Year :	101	100-Year :	500-Year
nation	: (feet)	: Identification :	(ft.) N.G.V.D.:	Flood	Flood	Flood	Flood
٠.	78+65	Divided Flow	5021.0	5024.0	5024.0	5024 -0	5024.0
				150	150	150	150
×	85+50	Divided Flow	5028.5	5031.0	5031.0	5031.0	5031.0
				150	150	150	150
Ц	93+50		5031.2	5032.7	5033.2	5033.4	5033.7
				1140	2780	3840	5940
Σ	101+50	-	5031.3	5034.5	5035.3	5035.6	5036.0
				1140	2780	3840	5940
Z	107+80		5033.0	5037.2	5037.9	5038.0	5038.4
				1140	2780	3840	5940
0	115+90		5035.0	5041.0	5041.7	5042.0	5042.4
				1140	2780	3840	5940
Ь	130+40		5041.2	5043.2	5044.5	5044.9	5045.5
				1140	2780	3840	5940
0	131+40	County Road	5041.3	5046.2	5048.3	5048.6	5048.7
				1140	2780	3840	5940
ĸ	132+40		5041.5	5048.2	5048.7	5048.9	5049.3
				1140	2780	3840	5940

plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood road crossings, upstream diversions, etc. 1

Cross	: Stationing		••	••	Crest-Elev	Crest-Elevation Feet National Geodetic Vertical	tional Geode	etic Ve	tical
Section	: From Lower	••	••	Stream Bed :	Da	Datum, and Peak Discharge c.f.s.	Discharge	c.f.s.	
Desig-	: Study Limit	••	••	Elevation :	10-Year	: 50-Year	: 100-Year	ar :	500-Year
nation	: (feet)		Identification :	(ft.) N.G.V.D.:	Flood	Flood	Flood		Flood
v:	143+80			5045.2	7,6705	5050.0	5050		8 0505
					1140	2780	3840		5940
E	151+50			5049.0	5053.7	5055.2	5055.6		5056.1
					1140	2780	3840		5940
n	159+50		•	5053.3	5058.6	5061.5	5062.0		5062.5
					1140	2780	3840		5940
Λ	167+70			5057.5	5064.2	5066.0	5066.4		5066.8
					1140	2780	3840		5940
Μ	176+50		3	5061.0	5067.4	5.069.7	5070.4		5070.9
		-			1140	2780	3840		5940
×	184+90			5063.0	5070.0	5073.2	5073.7		5074.7
					1.140	2780	3840		5940
Y	193+90			5069.7	5074.2	5076.9	5078.2		5079.0
					1140	2780	3840		5940
2	195+90	ర	County Road	5071.2	5076.2	5079.7	5081.9		5082.2
					1140	2780	3840		5940

plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood road crossings, upstream diversions, etc.

Cross	: Stationing From Lower		Stream Bed :	Crest-Elevatio	tion Feet Nati	Crest-Elevation Feet National Geodetic Vertical Datum, and Peak Discharge c.f.s.	Vertical
Desig- nation	Study Limit (feet)	: Identification :	Elevation (ft.) N.G.V.D.:	10-Year : Flood :	10	: 100-Year : Flood	500-Year Flood
AA	197+90		5072.7	5078.7	5082.4	5082.5 3840	5082.9
AB	205+50		5079.2	5081.3	5082.5	5082.6 3840	5083.0
AC	212+80		5085.9	5088.1	5088.5 2780	5089 .1 3840	5089.5
AD	223+20		5091.6	5094.8	5094.9	5094.9 3840	5094.9
AE	231+85		5099.8	5100.9	5101.5 2780	5101.5 3840	5101.5
AF	239+65		5095.9	5103.5	5104.3	5104.4	5104.4
AG	247+05		5098.1	5104.7 1140	5106.i 2780	5106.6 3840	5107.7
АН	253+45		5100.0	5105.7 1140	5108.2	5109.4 3840	5110.3
AI	255+15	County Road	5103.3	5107.1 1140	5109.2	5110.3 3840	5111.7
AJ	258+05	Confluence with Indian Creek	5106.5	5111.5	5112.5	5112.8	5113.7

Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to road crossings, upstream diversions, etc. 17

Cross	: Stationing : From Lower		: Stream Bed :	Crest-Elevati Datum	ion Feet Nation, and Peak Di	Crest-Elevation Feet National Geodetic Vertica Datum, and Peak Discharge c.f.s.	Vertical S.
Desig- nation	: Study Limit : (feet)	: Identification	: Elevation : (ft.) N.G.V.D.:	10-Year : Flood :	50-Year : Flood :	100-Year Flood	: 500-Year : Flood
AJ	258+05	Begin Reach 2	5106.5	5111.5 1140	5112.5 2780	5112.8	5113.7 5940
BA-East	267+05	Divided Flow	5113.2	5115.8 890	5116.3 2530	5116.6	5117.0 5690
BB-East	280+45	Divided Flow	5120.5	5121.8 890	5122.6 2530	5122.8 3590	5123.4 5690
BC-East	288+85	Divided Flow	5123.5	5126.8 890	5127.3	5127.5	5127.9
BD-East	297+75	Divided Flow	5126.0	5128.4 890	5129.4 2530 .	5129.7	5130.3 5690
BE-East	BE-East <u>2</u> / 321+75		5135.0	5138.8 900	5139.1 1670	5139.2	5139.5

1

This is the upstream end of a divided flow reach. Water surface elevations maybe slightly different than shown on the main channel at cross section BE. 2/

tical	500-Year Flood	5113.7	5114.0	5122.0	5126.5	5131.7	5140.0	5147.5	5149.6
c Ver									
Crest-Elevation Feet National Geodetic Vertical Datum, and Peak Discharge c.f.s.	100-Year Flood	5112.8 3840	5114.0	5122.0 250	5126.5	5131.7	5139.7 2140	5147.5	5149.2
Natic ak Di									
ion Feet m, and Pe		5112.5	5114.0	5122.0	5126.5	5131.7	5139.5	5147.5	5148.9 1670
levatior Datum,									
Crest-E	10-Year Flood	5111.5	5114.0	5122.0 250	5126.5	5131.7	5139.1	5147.1	5148.9
•• ••									
Stream Bed	Elevation (ft.) N.G.V.D.:	5106.5	5110.0	5120.1	5123.8	5129.5	5135.0	5146.1	5146.1
	: Identification	Begin Reach 2	Divided Flow	Divided Flow	Divided Flow	Divided Flow		County Road	
p0 ri	it								
Stationing From Lower	Study Limit (feet)	258+05	268+85	281+65	292+75	303+35	316+35	334+15	335+95
Cross	Desig- nation	AJ	BA	BB	ВС	BD	BE	BG	ВН

plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood road crossings, upstream diversions, etc. 1

Cross		Stationing		••	: Crest-Elev	ration Feet Na	Crest-Elevation Feet National Geodetic Vertical	c Vert	ical
Section	••	From Lower	••	: Stream Bed	••	itum, and Peak	Datum, and Peak Discharge c.f.s.	· S	
Desig-	••	Study Limit		: Elevation	: 10-Year	: 50-Year	: 100-Year		500-Year
nation	••	(feet)	: Identification	: (ft.) N.G.V.D.:	V.D.: Flood	Flood	: Flood	••	Flood
BI		339+55		5146.3	5149.9	5150.6 1670	5150.9		5151.4
BJ		341+05		5147.5	5150.0 900	5150.7 1670	5151.0 2140		5151.6
ВК		343+85	Interstate 25	5147.5	5150.1 900	5150.8 1670	5151.2		5151.8
BL		346+35	Confluence with Coal Creek	5149.0	5152.9	5153.6	5153.9		5154.4

Table page 9

Cross		Stationing	••	••	: Crest-Elev	ration Feet Na	Crest-Elevation Feet National Geodetic Vertical	c Ver	tical
Section	• •	From Lower	••	: Stream Bed	: Da	atum, and Peak	Datum, and Peak Discharge c.f.s.	· S ·	
Desig-	••	Study Limit	••	: Elevation	: 10-Year	: 50-Year	: 100-Year	•••	500-Year
nation	••	(feet)	: Identification	: (ft.) N.G.V.D.	.: Flood	Flood	Flood	••	Flood
BL		346+35	Begin Reach 3	5149.0	5152.9	5153.6	5153.9		5154.4
					900	16/0	214()		3100
CA		352+85		5151.5	5155.4	5156.2	5156.5		5156.9
					480	920	1170		1690
CB		359+85		5154.0	5156.9	5157.9	5158.3		5159.0
1					480	920	1170		1690
22		366+85		5156.3	5160.7	5162.0	5162.6		5163.3
					480	920	1170		1690
CD		373+35		5159.0	5162.5	5164.0	5164.7		5165.6
1					480	920	1170		1690
CE		379+35		5161.0	5165.0	5166.2	5166.8		5167.8
					480	920	1170		1690
CF		384+35		5166.1	5169.0	5170.4	5171.0		5172.0
					480	920	1170		1690

Cross		Stationing			Crest-Ele	Crest-Elevation Feet National Geodetic Vertical	ational Geo	detic V	ertical
Section	••	From Lower	••	: Stream Bed :	D	Datum, and Peak Discharge c.f.s.	k Discharge	c.f.s.	
Desig-	••	Study Limit		: Elevation :	10-Year	: 50-Year	: 100-Year	(ear :	500-Year
nation		(feet)	: Identification	: (ft.) N.G.V.D.:	Flood	Flood	Flood	: poc	Flood
90		390+55		5169.0	5172.6	5174.0	5174.6	9.	5175.7
СН		395+55		5170.0	5174.0	5175.4	5175.9	6.	5176.8
CI		400+85	County Road	5177.9	5179.5	5180.0 920	5180.3	6	5180.7

Table page 11

Stationing	ning	** •		200	Crest-Eleva	ation Feet Na	Crest-Elevation Feet National Geodetic Vertical	c Ver	tical
wer :	••			Stream bed : Elevation :	10-Year	: 50-Year	. 50-Year : 100-Year :	S.	500-Year
(feet) : Identif	: Identif	Identif	Identification	(ft.) N.G.V.D.:	Flood	: Flood	: Flood	••	Flood
400+85 Begin Reach 4	Begin Reac	Begin Reac	h 4	5177.9	5179.5	5180.0 920	5180.3		5180.7
408+85				5178.8	5182.5	5183.5	5183.9		5184.7
413+70				5180.8	5183.5	5184.5 920	5185.0 1170		5185.8
414+00 Colorado & Southern Railroad Bridge	Colorado & S Railroad Br	Colorado & Railroad Br	Southern	5180.8	5183.6	5184.7 920	5185.1 1170		5185.9
416+00				5180.7	5184.5	5185.2 920	5185.6		5186.5
419+00				5181.9	5187.3	5188.7	5189.2		5190.1 1690
423+50				5184.4	5188.9	5190.3 920	5190.9 1170		5191.8

plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood road crossings, upstream diversions, etc.

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Cross	: Stationing		••	Crest-Elev	ation Feet Na	Crest-Elevation Feet National Geodetic Vertical	Vertical
Section	From Lower	••	: Stream Bed :	Da	itum, and Peal	Datum, and Peak Discharge c.f.s.	S.
Design	: Study Limit	•••	: Elevation :	10-Year	: 50-Year	: 100-Year	: 500-Year
nation	: (feet)	: Identification	: (ft.) N.G.V.D.:	Flood	Flood	Flood	Flood
Hu	00+267		5185.4	5189.7	5191.2	5191.8	5192.9
				480	920	1170	1690
Tu	06+667		5187.4	5191.6	5193.0	5193.7	5194.8
7.7				480	920	1170	1690
10	07+087	Cleveland Ave.	5187.4	5191.6	5193.0	5193.7	5194.8
2				480	920	1170	1690

Table page 13

Cross	3	Stationing				Crest-Ele	vation	Feet Nati	Crest-Flevation Feet National Geodetic Vertical	C Ver	rical
Section	Fr	From Lower	• ••		: Stream Bed :	a	atum, a	nd Peak D	Datum, and Peak Discharge c.f.s.	. S .	
Desig-	: St	Study Limit	••		: Elevation :	10-Year	. 50	50-Year	100-Year	••	500-Year
nation		(feet)		Identification	: (ft.) N.G.V.D.:	Flood		Flood	Flood	••	Flood
DJ	430	430+40		Begin Reach 5	5187.4	5191.6	51.	5193.0 920	5193.7		5194.8
EA	433	433+40			5188.0	5192.9	519	5194.4 920	5195.1 1170		5196.3
F. B.	437	437+60			5189.5	5193.7 480	51	5195.1 920	5195.8		5196.9
EC	645	443+00		·	5194.8	5198.5	51.	5199.6 920	5200.1 1170		5200.9
FF	447	447+15			5197.3	5201.0	520	5202.3 920	5202.9		5203.9
EG	451	451+25			5198.0	5201.9 480	52(	5203.2 920	5203.8 1170		5204.8
ЕН	457	457+25			5198.6	5203.1 480	520	5204.7 920	5205.4		5206.5

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## From Lower : Stream Bed : Datum, and Peak Disch	Cross		Stationing				: Crest-El	evation	Feet Nat	tional	Crest-Elevation Feet National Geodetic Vertical	Ver	tical
: Study Limit : : Elevation : 10-Year : 50-Year : 1 (feet) : (feet) : (ft.) N.G.V.D.: Flood :	Section	• •	From Lower	• •		: Stream Bed	• •	Datum, a	nd Peak	Disch	narge c.f.	s °	
: (feet) : Identification : (ft.) N.G.V.D.: Flood : Flood : 464+25	Desig-		Study Limit	• •		: Elevation	: 10-Year	: 50	-Year		100-Year	••	500-Year
464+25       5204.5       5206.0         480       920         465+55       County Road       5207.1       5209.3       5209.9	nation	• • •	(feet)	: Ide	ntification	: (ft.) N.G.V.D.		·•	lood.		Flood	••	Flood
464+25       5202.4       5206.0         480       920         465+55       County Road       5207.1       5209.3       5209.9													
465+55 County Road 5207.1 5209.3 5209.9	Į,		464+25			5202.4	5204.5	52	0.90	ĸ)	5206.6		5207.7
465+55 County Road 5207.1 5209.3 5209.9	1						480	6	20	1	1170		1690
465+55 County Road 5207.1 5209.3 5209.9 480 920													
480 920	Ħ		465+55	County	r Road	5207.1	5209.3	52	6.60	u)	5210.2		5210.7
	)						480	6	20		1170		1690

Table page 15

Cross	: Stationing			Crest-Ele	Crest-Elevation Feet National Geodetic Vertical	ational	Geodetic	Vert	ical
Desig- nation	Study Limit: (feet)	 Identification	Elevation : (ft.) N.G.V.D.:	10-Year Flood	: 50-Year : Flood	10	100-Year Flood		500-Year Flood
FJ	465+55	Begin Reach 6	5207.1	5209.3	5209.9 850	5.	5210.2		5210.7 1530
FA	473+85		5209.7	5213.6 470	5214.6	5.	5215.0		5215.8 1530
FB	479+85		5212.7	5215.7 470	5216.5 850	5.	5217.0 1080		5217.7
FC	487+85		5218.4	5222.2	5223.4 850	5.	5223.9 1080		5224.8
FD	496+65	Upper Study Limit	5219.1	5224.0 470	5225.4 850	5.	52226 • 1 1080		5227.5 1530

plain. However, flood elevations in the outer portions of a cross section may differ from the primary channel due to Flood elevations pertain to the primary channel and usually remain constant in a lateral direction across the flood road crossings, upstream diversions, etc.  $\frac{1}{1}$ 

Cross	: Stationing		. Correct Root	Crest-Eleva	Crest-Elevation Feet National Geodetic Vertical	onal Geodetic V	ertical
Section Designation	: From Lower : Study Limit : (feet)	Identification	Stream bed: Elevation: (ft.) N.G.V.D.:	10-Year Flood	SO-Year : 100-Year   Flood	1scharge c.f.s. 100-Year : Flood :	500-Year Flood
BL	346+35	Begin Reach 7 Coal Creek	5149.0	5152.9	5153.6 600	5153.9 830	5154.4
GA	352+25		5155.3	5156.0	5156.3 600	5156.5 830	5156.8
GB	356+75		5156.0	5158.2	5158.7 600	5158.9 830	5159.3 1300
29	360+95		5159.3	5159.9	5160.3 600	5160.5	5160.8
GD	366+55		5165.2	5165.9	5166.2	5166.3 830	5166.3
GF.	372+75		5167.9	5168.4	5168.7	5168.8	5169.2
GF	379+15		5172.1	5172.7	5172.8 600	5172.9 830	5173.0 1300
99	385–25		5175.7	5176.4	5176.8	5176.9 830	5177.2
GH	391+45	County Road	5181.5	5181.9	5182.1	5182.1 830	5182.3

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Cross	: Stationing : From Lower		Stream Bed	Crest-Eleva	tion Feet Nati um, and Peak D	Crest-Elevation Feet National Geodetic Vertical Datum, and Peak Discharge c.f.s.	ertical
Desig- nation	Study Limit (feet)	t : : Identification	: Elevation : (ft.) N.G.V.D.:	10-Year Flood	50-Year : Flood :	100-Year Flood	500-Year Flood
СН	391+45	Begin Reach 8	5181.5	5181.9 230	5182.1 600	5182.1 830	5182.3
НА	398+15		5180.5	5183.4	5184.3	5184.6 830	5185.0
нв	406+15		5181.8	5184.3	5185.6	5186.0	5186.6
НС	409+15		5182.6	5185.2	5186.4	5186.8	5187.6
Н	413+95	Roosevelt Ave.	5189.0	5189.9	5190.3	5190.5	5190.8
HE	417+85		5189.2	5191.3	5192.1 600	5192.4	5192.9
HF	423+45	Cleveland Ave.	5199.0	5199.5	5199.7	5199.8	5199.9
HG	427+05		5199.6	5201.1	5201.6	5201.8	5202.2

ertical	500-Year Flood	5203.3	5206.4	5209.2	5210.6 1300	5214.8	5216.8 1300	5218.9
al Geodetic V	100-Year Flood	5203.1 830	5206.4	5208.9 830	5210.3 830	5214.3 830	5216.6 830	5218.8
Crest-Elevation Feet National Geodetic Vertical Datum, and Peak Discharge c.f.s.	50-Year : Flood :	5202.9	5206.4 600	5208.8 600	5210.2 600	5214.2 600	5216.5 600	5218.7 600
Crest-Elevati	10-Year : Flood :	5202.3	5206.3 230	5208.7 230	5209.8 230	5214 •0 230	5216.2 230	5218.6 230
Stream Bed :	Elevation : (ft.) N.G.V.D.:	5201.7	5206.0	5208.1	5208.8	5213.5	5215.5	5218.1
	: Identification :			Grant Ave.				County Road
Stationing : From Lower :	Study Limit : (feet) :	430+45	433+85	437+25	440+85	, 447+65	450+90	451+20
Cross :	Desig- :	НН	HI	нл	НК	HL	HM	NH

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Cross	••	Stationing			••	Crest-El	evati	on Feet Na	tion	Crest-Elevation Feet National Geodetic Vertical	Ver	ical
Section	••	From Lower		••	Stream Bed :		Datum	, and Peak	Disc	Datum, and Peak Discharge c.f.s.	s.	
Desig-	••	Study Limit		••	Elevation :	10-Year		50-Year		100-Year	••	500-Year
nation	••	(feet)	: Identification		(ft.) N.G.V.D.:	Flood		Flood		Flood		Flood
HN		451+20	Begin Reach 9		5218.1	5218.6		5218.7		5218.8		5218.9
19						430		800		1030		1500
IA		452+25			5219.0	5219.7		5219.9		5220.0		5220.1
						430		800		1030		1500
IB		457+55			5219.8	5220.5		5220.7		5220.8		5221.0
						430		800		1030		1500
IC		462+75			5221.4	5221.8		5221.9		5222.0		5222.2
						430		800		1030		1500
ID		470+25	,		5225.0	5225.9		5226.1		5226.3		5226.4
						430		800		1030		1500
IE		478+55	Upper Study Limit		5229.0	5229.5		5229.7		5229.8		5230.0
-						430		800		1030		1500





